

Turfpro LLC

2/19/2024

PO Box 386, Norwalk, CT 06852

Commissioners,

My name is Yuriy Strus. I am the owner of Turfpro LLC, and we specialize in land development and site work. For ten years I was assistant of head greens keeper at Silvermine Golf Club in Norwalk, CT. I have substantial experience, and equipment to work in tight spaces and with minimal impact on the surroundings, and more importantly sub-soil.

I was asked to assess the site work, with particular regard to the driveway construction at 0 Mountain Road.

Looking at the plan, we realize that access/easement to the site is somewhat challenging and will require a careful approach and planning of all logistics involved. It is obvious that this project will take extra time and funds, yet it is feasible.

The construction of the driveway, I see like a domino effect: you go step by step, starting from the entrance point. Starting with a mini-excavator and dump truck, taking fill away and delivering aggregates, gravel/process, and where necessary installing the I shaped retaining walls, along with culverts. While the staging area is limited, it is possible to store/transfer aggregates that we will need to stay within the foot print of the site and not disturb neighboring parcels.

In general, the test holes did not expose much ledge, but in case we find any, we would clip rock breaker on our equipment and load ledge with smaller excavator. It seems that most of the work can be done with smaller equipment such as, low ground pressure, track skid steer loader and mini excavator. To set the culverts we will need bigger size, zero swing excavator that does not exceed DOT over width regulation of 8'6" in order to go through the tighter areas of the driveway. In a project like this, I do envision a good amount of manual labor, and in order to pave the narrow area we will be using our smaller paving box.

The same applies to the wetland crossing. We will build the road in foot by foot, structure by structure, and will follow town and state regulation for construction and erosion control.

To sum it up, I see the parcel as challenging, yet buildable.

Thank you,

Yuriy Strus

FAIRFIELD COUNTY ENGINEERING, LLC
CIVIL ENGINEERS
60 WINFIELD ST.
NORWALK, CONNECTICUT 06855
(203) 831-8005 FAX: (203) 831-8006
E-mail to: wayne@fairfieldce.com

February 9, 2024

Please see the following in response to comments made by the public and commissioners at the February 8, 2024 Wetlands Commission meeting, and the Engineering department's letter dated February 16, 2024.

Temporary construction easements are not needed, as there are no proposed structures or grading off the accessway.

The proposed retaining walls are to be built with concrete block and therefore will not require any formwork or over digging. There is no drain pipe behind the wall, so the rear of the wall can be placed within an inch of the property line if necessary. Weep holes will be placed at 8 foot intervals.

Along the lengths where the rear of the wall is within one foot of the property line there will be no stone placed behind the wall. The proposed footings are robust and ample to provide a stable wall, given the proposed walls 2 to 3 foot height.

The tree stumps can be ground and removed to the property line where necessary. A stump grinding contractor has provided a statement affirming this.

A small paving box can be used to place the asphalt in the choke point, and spread by hand where necessary. The proposed driveway is at grade at the choke point and will not require excavation, fill or grading at that point. The rest of the accessway widens to where paving is not an issue.

Given the details above, the applicant has provided reasons as to why temporary easements are not needed. The town has simply expressed an opinion that they are, with no reason provided as to why, that can be responded to.

The consultant claimed to observe a soil type (gravel or sand) other than shown on the test hole data. As the holes were completely backfilled at the time of their visit, it is puzzling how this determination can be made. Regardless, the purpose of the holes was to determine the restrictive layers at specific locations, not chronicle soil type.

Test holes were dug at the exact locations of each of the proposed concrete gallery sets, validating the required 24" vertical clearance to ledge and the design. Out of 19 holes dug in the accessway and main lot, 15 had ledge 48" or deeper; some found no ledge to a depth of 5 or 6 feet.

The comment that infiltrators can only be used in certain types of soils is false. Having designed and seen built hundreds of retention systems, they are placed in all types of soils. The relevant qualities are depth to restrictive layer(s) and percolation rates, not color or granularity of the soil.

Again, the Town of Wilton has *no prohibition* in placing infiltrators in Hydrologic Group “D” soil. This is also a moot point as the majority of the soil on the site is Group “B”, per the USDA Natural Resources Conservation Service (NRCS) website. Having reviewed the analyses for the accessway and the main lot, Engineering tacitly agrees.

Engineering has also specifically stated the requirement for a 24” vertical separation from ledge to retention system in their commentary letter dated February 7, 2024. *That is the standard in the Town of Wilton.*

The comment that the proposed retention system “doesn’t provide infiltration” is broadly false. Again, having designed and seen hundreds of these types of systems installed and functioning, the design as proposed *is* the way stormwater runoff is properly mitigated to meet MS4 and the Town of Wilton requirements in my professional opinion. Again, Engineering has seen and reviewed these plans multiple times and has had no objection so broad and basic as that.

It was mentioned that a representative from Engineering had to be present at the soil tests. This is also false. That is not a requirement – nor provided – in the Town of Wilton. This is the normal practice in all towns in lower Fairfield County, with the exception of Westport. (Even there I have personally experienced occasions where there was no personnel available to witness, and the soil test occurred without the project being held up.) It is generally understood that a Professional Engineer has no reason or interest in mis-reporting soil test hole results.

The manufacturer of the box culvert confirms that a cut off wall is not needed for their installation. The 12” stone base is their specification.

The manufacturer also verifies that the box culverts weigh 13,000 pounds, not 17,000 pounds as claimed. A contractor confirms that several models of mini excavators can easily maneuver in the accessway and can lift, carry and place these units.

The box culverts are the most minimal feasible way to provide a vehicular crossing of the wetlands. A bridge is not only cost prohibitive, but not possible to construct in this setting. To be less invasive than the box culverts, there could be no abutment or pier in the wetlands. The excavation and activity needed to construct such structure(s) would be more disruptive than the placing of nine units of box culvert. To do this the bridge must span 160 feet, and thus need 160 foot long structural members. Such long objects require a crane to lift into place, and there is no room for a machine that large in the accessway. It is likely additional trees would have to be removed just to move it into position, even if the accessway were wide enough.

The only other possibility is to place the driveway on an earthen berm, with a few pipes acting as the culvert. This would require far more fill and disturbance in the wetland and near the vernal pool than the box culverts, and not allow the opening space for animal migration that the box culverts do.

It was suggested that the drainage had to connect to existing drainage infrastructure and that the water “had to have a place to go to”. There is no drainage infrastructure to connect to within a reasonable distance. This would also be diametrically opposed to the Town and MS4 goal of retaining and treating stormwater runoff on site.

The treatment train of deep sump catchbasins and infiltrators provides 92.5% TSS (Total Suspended Solids) removal from the runoff. Again, this is a widely accepted practice or fact in all towns in the area. This exceeds the Town of Wilton requirement of 80% TSS removal.

It was commented that the galleries would be full and not retaining [further] water when the overflow backs up to the catchbasin grates, as designed. We are stating what occurs during an *overflow* condition, which, of course occurs when the galleries are *full*. This obviously happens towards the end of large (25 Year) storms. The point is that the system as designed is *more* conservative than the HydroCAD model shows. The grates are the best location for the *overflow* to occur.

(It should also be noted that the HydroCAD model assumes no exfiltration at all. This is very conservative; the percolation tests on the main lot revealed a 1"/10 minute percolation rate.)

The gallery sets that are in the narrowest area of the accessway are in an area of excavation and not fill. They are proposed *below* the existing grade (i.e. not "mounded") and don't pose a great risk of bleedout to the lower property. Their proximity to property lines is unavoidable and not able to be further mitigated given the nature of the accessway and the Town's Stormwater management requirements. Should the Town waive or lessen the requirement perhaps some of the sets could be reduced in size or eliminated.

There has been concern over heavy machinery on the porous asphalt. Again, this is a proposed driveway to a single family residence. It will not be seeing heavy traffic, only standard cars, SUVs and an occasional delivery truck. The daily traffic volume count will be in the single digits, not hundreds. Porous asphalt is used on single family residences in many towns. The Town of Greenwich encourages it, and I have seen several approved there; two on projects that I performed drainage design, and one in Westport. I have also seen porous asphalt on entire commercial parking lots, not just the parking areas.

It is true that it requires maintenance, but so do all other drainage items; catchbasins, manholes and regular asphalt. To ensure the maintenance of the drainage items, municipalities can require a Maintenance and Operations plan be mentioned and filed in the Land Records. That could be done in this case.

It should be noted that the porous asphalt is proposed only in the area of the wetland and vernal pool, not the entire length of the driveway. In this sensitive area the benefits of the porous asphalt outweigh its negatives.

The box culverts are designed for HL 93 loading (72,000 pound tri axle vehicle). The Fire Marshall has signed off on the driveway.

Respectfully submitted,

Wayne D'Avanzo, P.E.

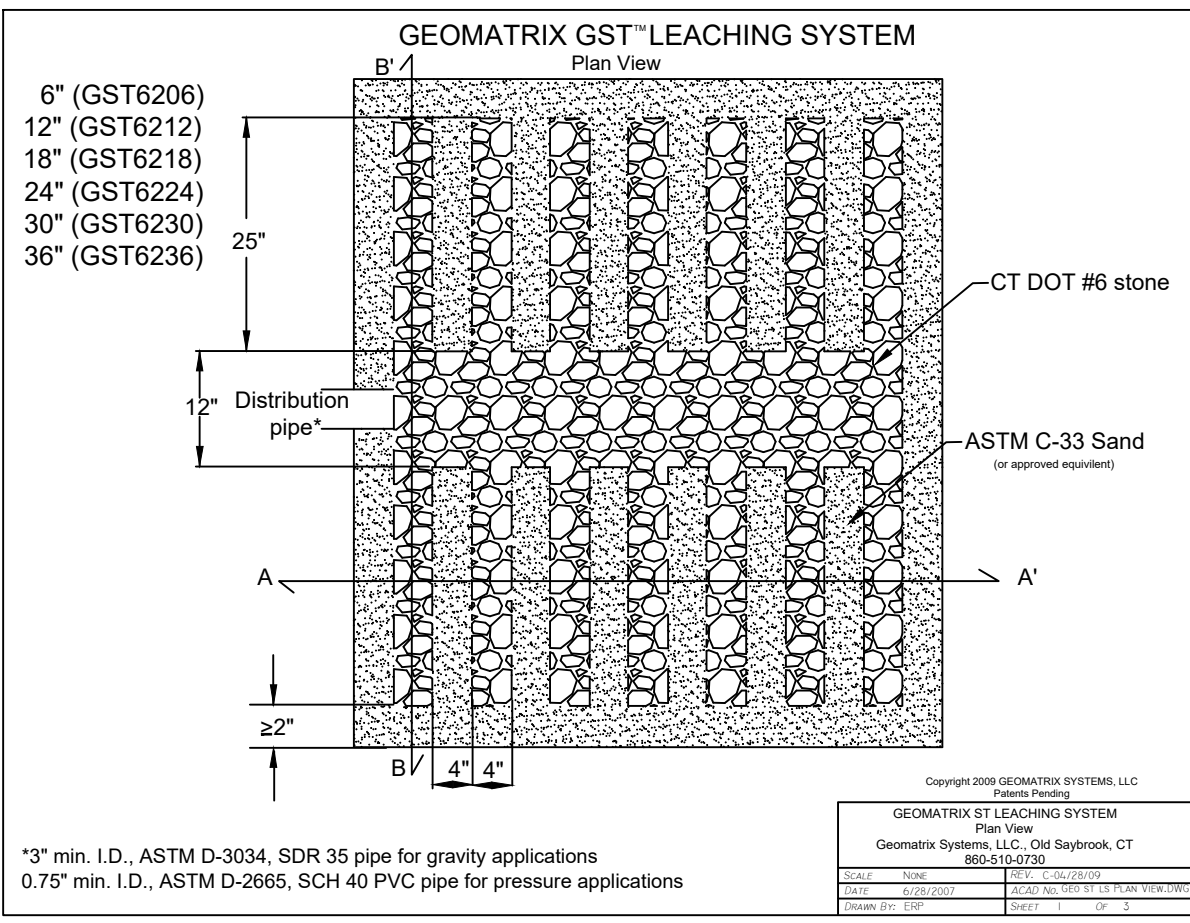
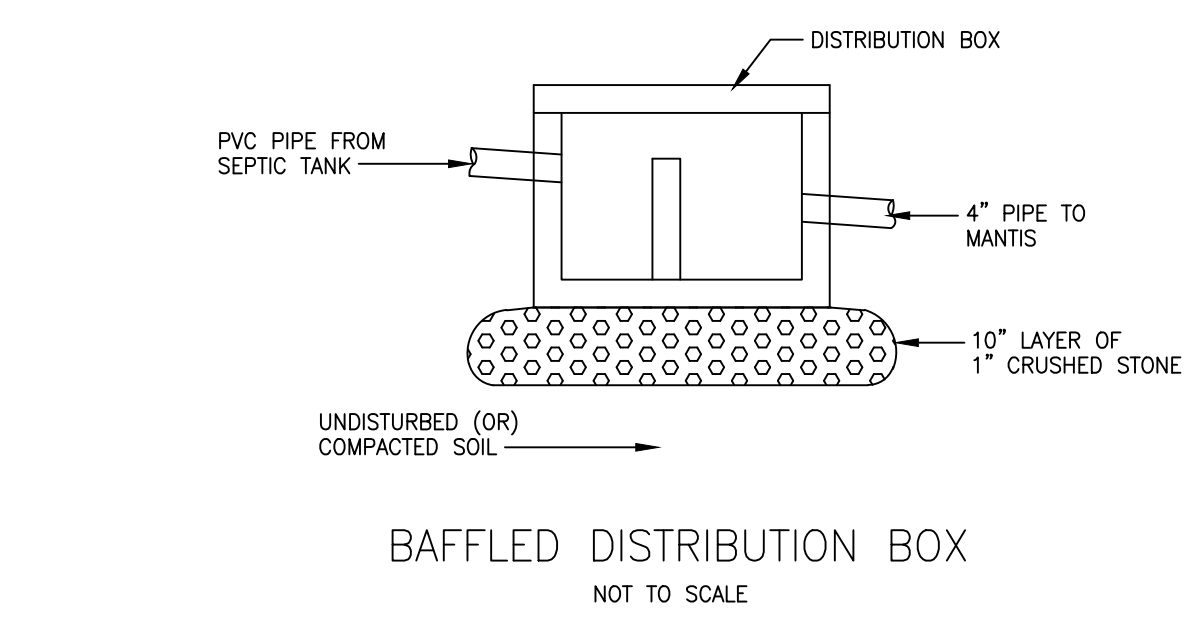
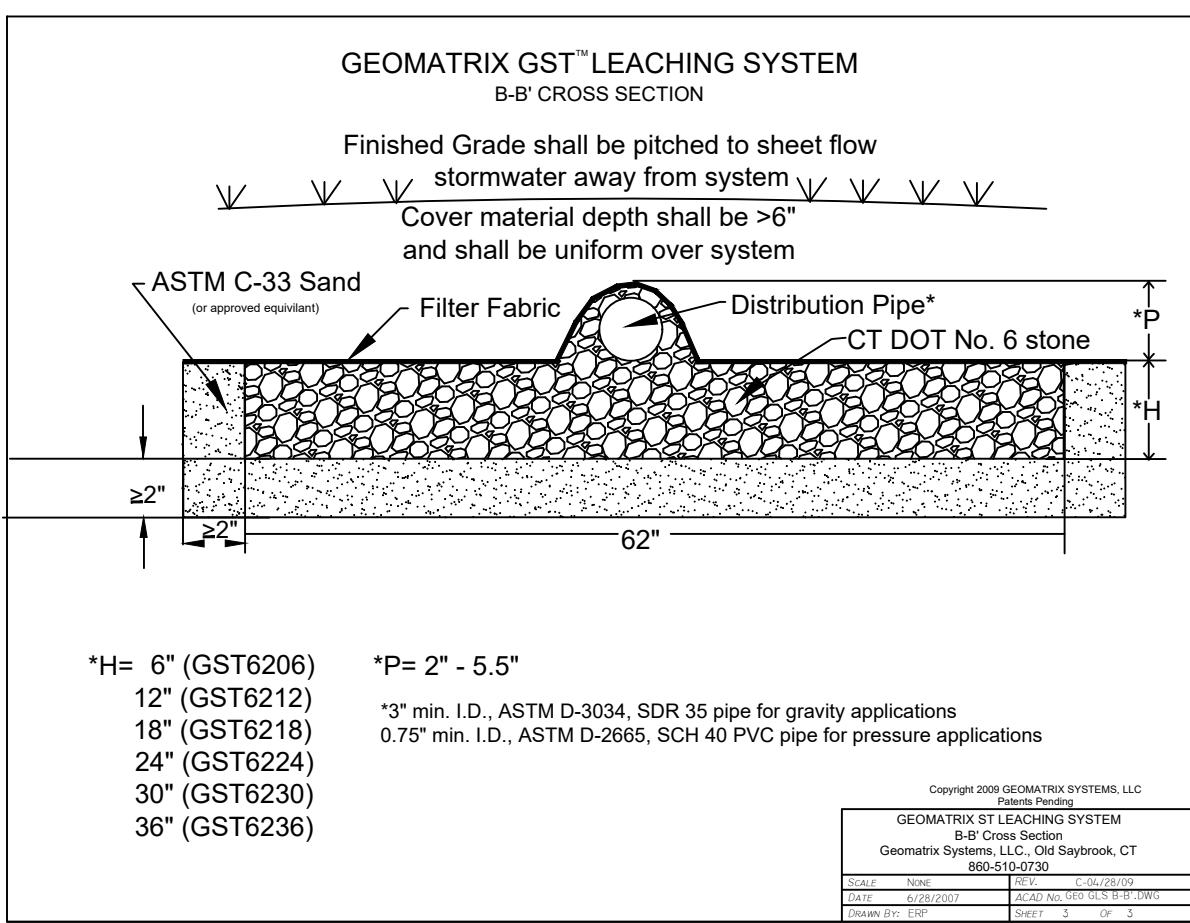
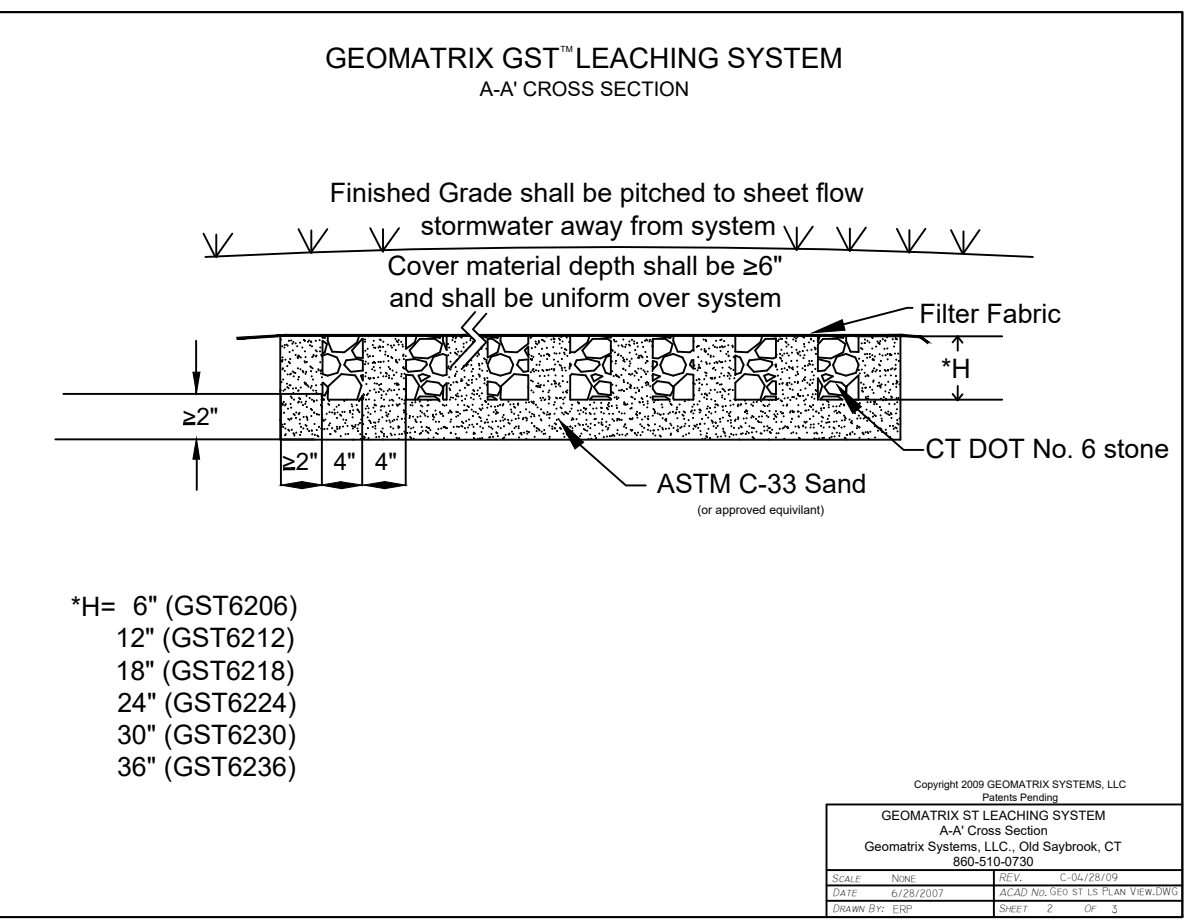
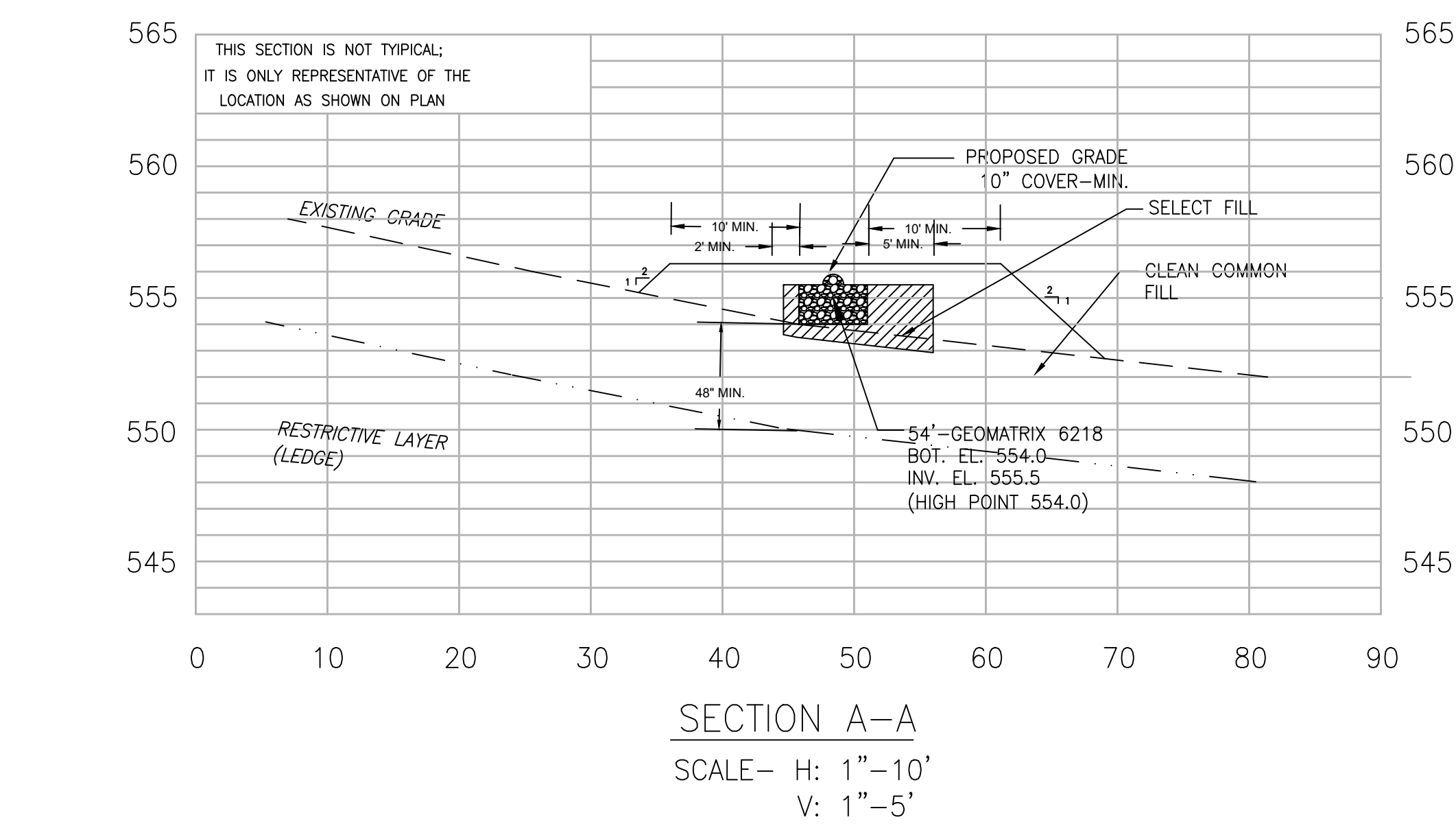
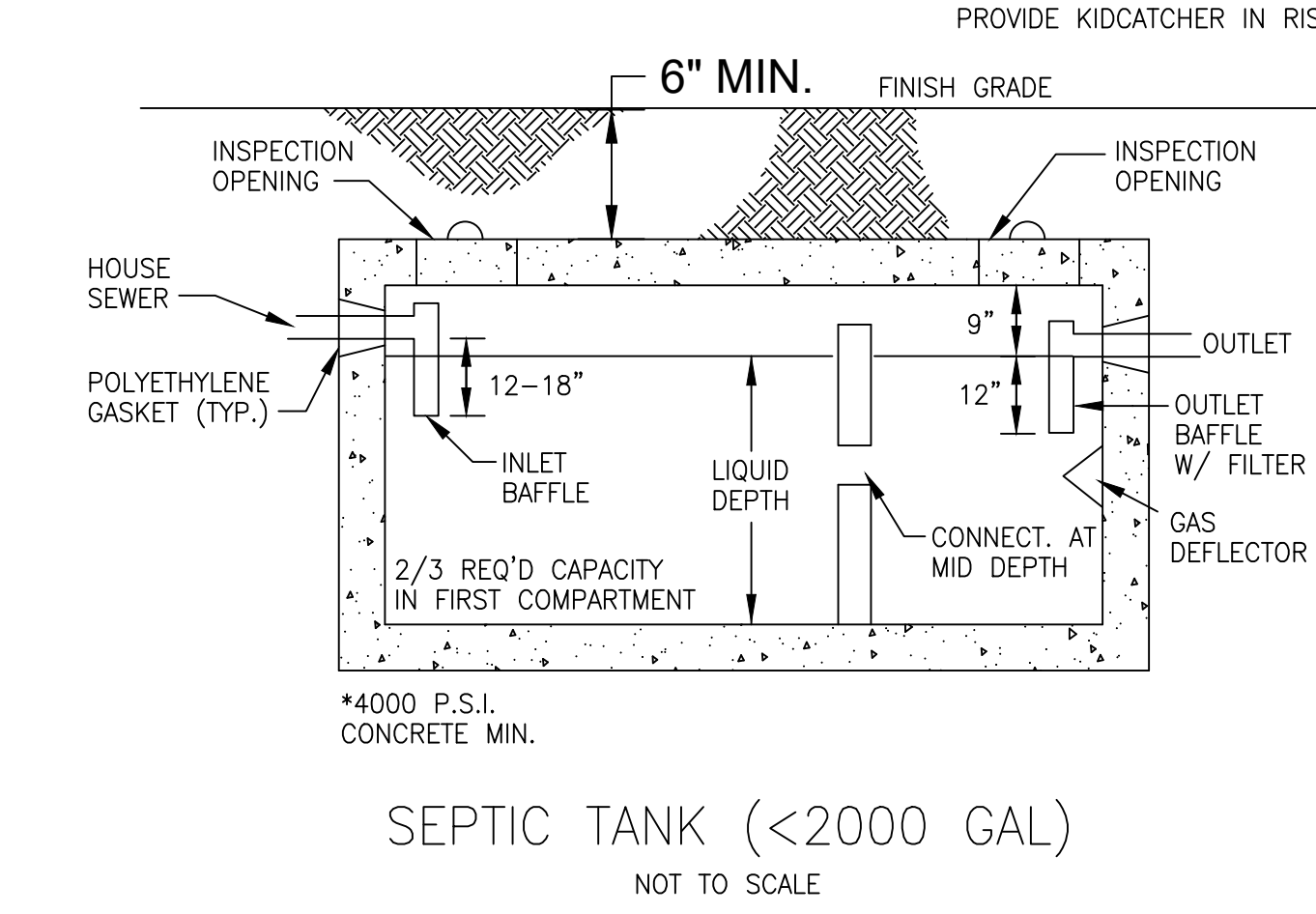
Principal

FAIRFIELD COUNTY ENGINEERING, LLC

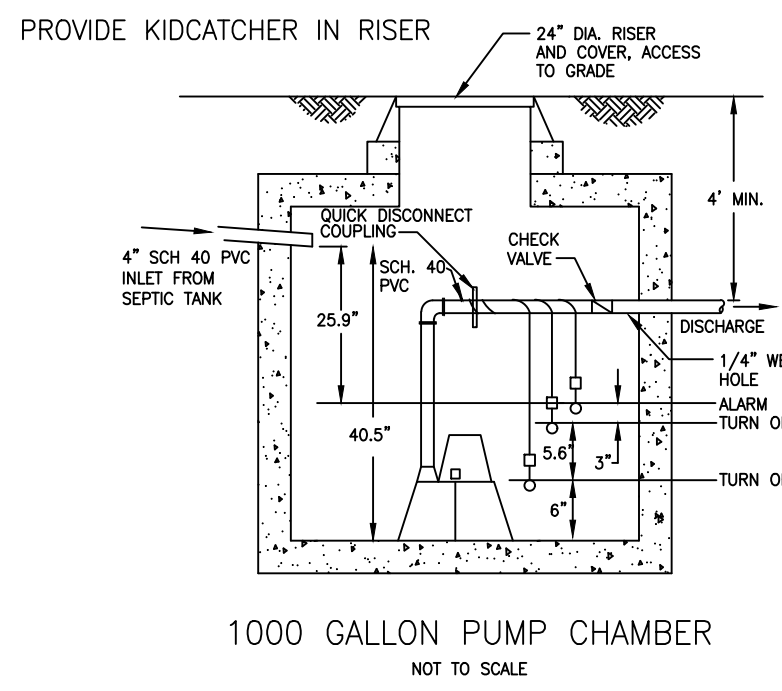
FCE Project #	2168	Date Performed:	6/21/2023
Client:	Old Driftway LLC		
Location:	0 Mountain Road, Wilton		
Observed by:	Wayne D'Avanzo		
Test Hole 1:	0-5" Topsoil 5-24" Tan Fine Gravel No Ground Water No Mottling Ledge @ 24" Roots to 20"		
Test Hole 2:	0-6" Topsoil 6-60" Tan Fine Gravel No Ground Water No Mottling Ledge @ 60" Roots to 24"		
Test Hole 3:	0-8" Topsoil 8-30" Tan Fine Gravel 30-52" Grey Fine Sand and Gravel No Ground Water No Mottling Ledge @ 52"		
Test Hole 4:	0-7" Topsoil 7-28" Tan Fine Gravel 28-49" Grey Fine Sand and Gravel No Ground Water No Mottling Ledge @ 49" Roots to 21"		
Test Hole 5:	0-6" Topsoil 6-29" Tan Fine Gravel 29-48" Grey Fine Sand and Gravel No Ground Water No Mottling Ledge @ 48" Roots to 26"		
Test Hole 6:	0-6" Topsoil 6-49" Tan Fine Gravel No Ground Water No Mottling Ledge @ 49" Roots to 36"		
Test Hole 7:	0-6" Topsoil 6-49" Tan Fine Gravel No Ground Water No Mottling Ledge @ 49" Roots to 40"		
Test Hole 8:	0-6" Topsoil 6-39" Tan Fine Gravel No Ground Water No Mottling Ledge @ 39" Roots to 29"		
Test Hole 9:	0-6" Topsoil 6-46" Tan Fine Gravel No Ground Water No Mottling Ledge @ 46" Roots to 26"		

SLOPE CALCULATIONS		
SLOPE LINE A: 3.5'/81.2'	4.3%	553.5 to 550.0
SLOPE LINE B: 3.7'/56.9'	6.5%	553.7 to 550.0
SLOPE LINE C: 5.7'/65.7'	8.7%	553.7 to 548.0
AVG. SLOPE 6.50%		

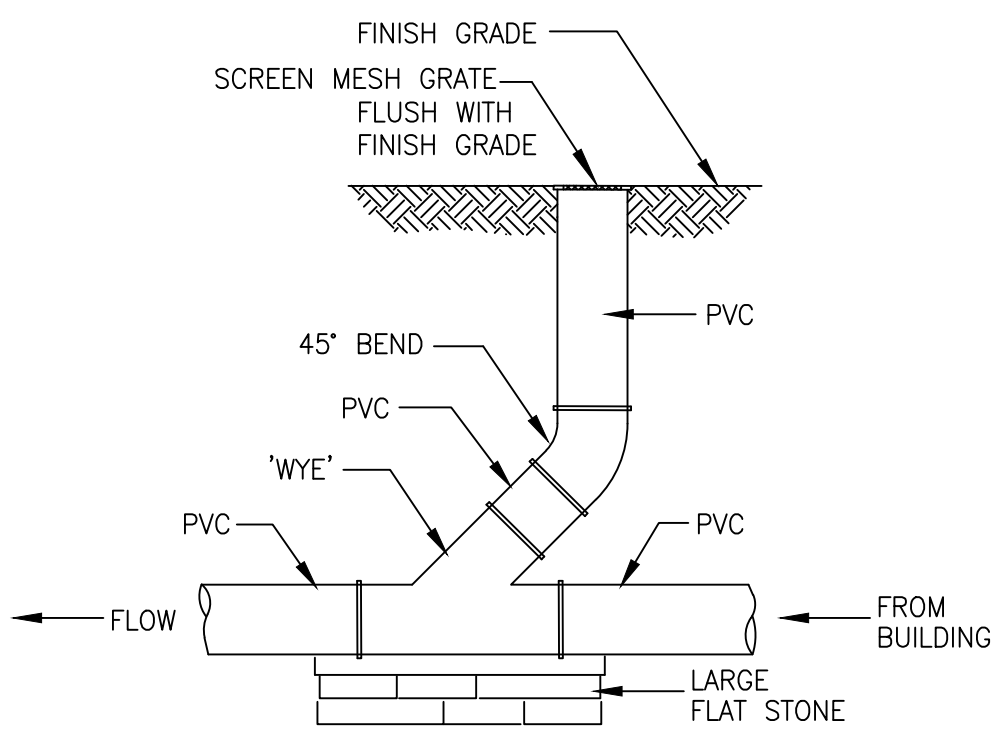
Conducted by:	Wayne D'Avanzo		Project:	2168
Location:	0 Mountain Road		Town:	Wilton
Client :	Old Driftway LLC		Date:	6/21/2023
Weather conditions prior to and during tests:				
Clear				
Single Lot:	X	Subdivision:		
Diameter of Hole:	8"	Depth of Hole:	19"	
PT-1			Design	
Pre-soak @ 9:30 AM			1"/10 Min.	
Time	Time Increment	Depth to Water	Drop in inches	Soil Percolation Rate Time to drop 1 inch
10:30 AM	----	6"	----	----
10:40 AM	10 Min.	14 1/2"	8 1/2"	1.2 Min.
10:50 AM	10 Min.	13 1/2"	4"	2.5 Min.
11:00 AM	10 Min.	13 1/8"	5 1/8"	2.0 Min.
11:10 AM	10 Min.	15 1/8"	2"	5.0 Min.
11:20 AM	10 Min.	8 7/8"	1 3/4"	5.7 Min.
11:30 AM	10 Min.	10 3/8"	1 1/2"	6.7 Min.
Single Lot:				
X		Subdivision:		
Diameter of Hole:	8"	Depth of Hole:	20"	
PT-2			Design	
Pre-soak @ 9:35 AM			1"/10 Min.	
Time	Time Increment	Depth to Water	Drop in inches	Soil Percolation Rate Time to drop 1 inch
10:35 AM	----	4"	----	----
10:45 AM	10 Min.	11"	7"	1.4 Min.
10:55 AM	10 Min.	15 1/4"	4 1/4"	2.4 Min.
11:05 AM	10 Min.	6 1/2"	3 1/2"	2.9 Min.
11:15 AM	10 Min.	8 3/4"	2 1/4"	4.4 Min.
11:25 AM	10 Min.	10 3/8"	1 5/8"	6.2 Min.
11:35 AM	10 Min.	11 7/8"	1 1/2"	6.7 Min.



FCE Project #	2168	Date Performed:	1/30/2023
Client:	Old Driftway LLC		
Location:	0 Mountain Road, Wilton		
Observed by:	Wayne D'Avanzo		
TH 10	0-5" Topsoil 5-60" Tan Fine Gravel No Ledge		
TH 11	0-5" Topsoil 5-36" Tan Fine Gravel Ledge @ 35"		

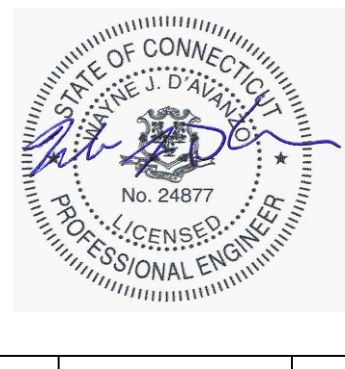


54' OF GEOMATRIX GST 6218 CAPACITY = 747.4 GALLONS
MAX. DOSE FOR 54' OF GEOMATRIX GST 6218 = 373.7 GALLONS/CYCLE
DOSE PROPOSED = 150 GALLONS PER CYCLE
PUMP CHAMBER CAPACITY = APPROX. 26.6 GALLONS/INCH DEPTH
PUMP 6" OFF BOTTOM = 159 GALLONS; CYCLE = 5.6" = 150 GALLONS;
691 GALLONS RESERVE 3 BEDROOMS X 150 GALLONS/DAY + 3 (75) = 675 GALLONS
LIBERTY 290 SERIES PUMP DELIVERS APPROX. 30 GALLONS/MIN.



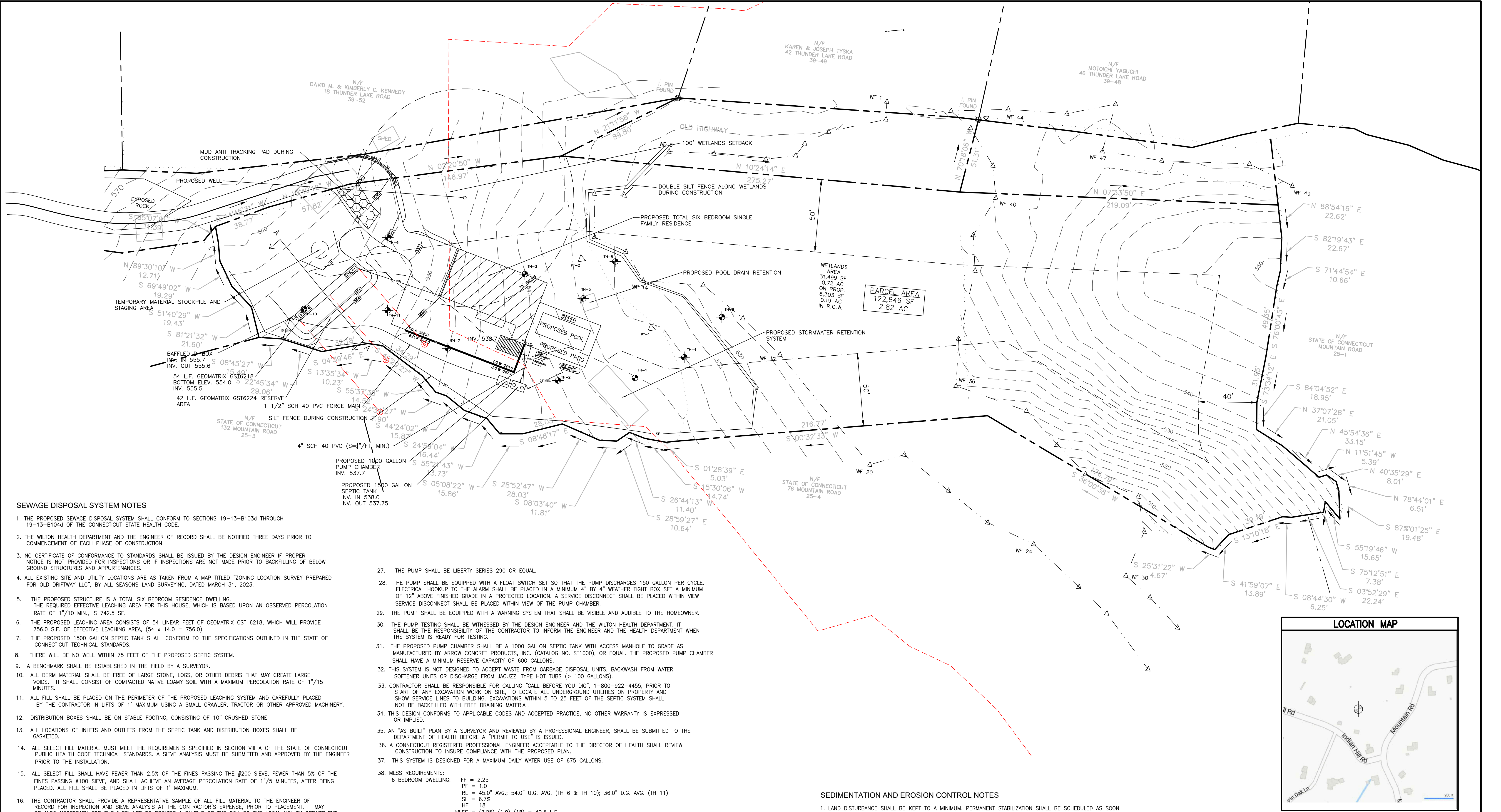
TYPICAL CLEANOUT
NOT TO SCALE

Geomatrix GST™ Leaching System			
Storage Volume and Dosing Recommendations			
Product Name	Dimensions (W x H)	Storage Volume Gallons per LF	Maximum Recommended Dosing Volume Gallons per LF
GST 3706	37" x 6"	3.05	1.53
GST 3712	37" x 12"	6.11	3.05
GST 3718	37" x 18"	9.16	4.58
GST 3724	37" x 24"	12.22	6.11
GST 3730	37" x 30"	15.27	7.64
GST 3736	37" x 36"	18.32	9.16
GST 6206	62" x 6"	4.62	2.31
GST 6212	62" x 12"	9.24	4.62
GST 6218	62" x 18"	13.84	6.92
GST 6224	62" x 24"	18.45	9.24
GST 6230	62" x 30"	23.06	11.53
GST 6236	62" x 36"	27.68	13.84
Stone and Sand Volume Guide			
Product Name	Amount of DOT No. 6 Stone Required Yards per Linear Foot	Amount of ASTM C-33 Sand Required Yards per Linear Foot	
GST 3706	0.16	0.16	
GST 3712	0.19	0.25	
GST 3718	0.22	0.32	
GST 3724	0.25	0.39	
GST 3730	0.28	0.46	
GST 3736	0.31	0.53	
GST 6206	0.20	0.20	
GST 6212	0.27	0.35	
GST 6218	0.35	0.46	
GST 6224	0.43	0.56	
GST 6230	0.50	0.66	
GST 6236	0.58	0.76	



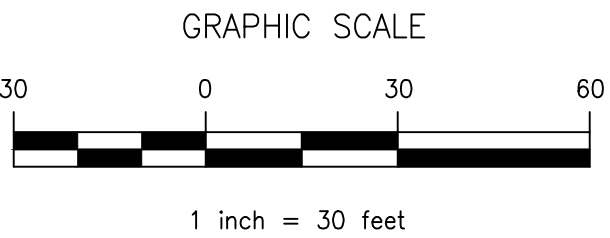
7-26-23 date	2168 project
2 OF 2 sheet	

REV. 2/19/24: SIX BEDROOMS.
REV. 1/15/24: TEST HOLES
REV. 12/16/23: PER WETLANDS.
OLD DRIFTWAY LLC
0 MOUNTAIN ROAD WILTON, CONNECTICUT
DETAIL SHEET
CIVIL ENGINEERS
FAIRFIELD COUNTY ENGINEERING L.L.C.
60 WINFIELD STREET, NORWALK, CONNECTICUT 06855 PH: (203) 831-8005 FAX: (203) 831-8006



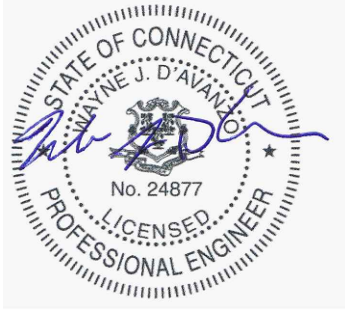
SEWAGE DISPOSAL SYSTEM NOTES

- THE PROPOSED SEWAGE DISPOSAL SYSTEM SHALL CONFORM TO SECTIONS 19-13-B103d THROUGH 19-13-B104d OF THE CONNECTICUT STATE HEALTH CODE.
- THE WILTON HEALTH DEPARTMENT AND THE ENGINEER OF RECORD SHALL BE NOTIFIED THREE DAYS PRIOR TO COMMENCEMENT OF EACH PHASE OF CONSTRUCTION.
- NO CERTIFICATE OF CONFORMANCE TO STANDARDS SHALL BE ISSUED BY THE DESIGN ENGINEER IF PROPER NOTICE IS NOT PROVIDED FOR INSPECTIONS OR IF INSPECTIONS ARE NOT MADE PRIOR TO BACKFILLING OF BELOW GROUND STRUCTURES AND APPURTENANCES.
- ALL EXISTING SITE AND UTILITY LOCATIONS ARE AS TAKEN FROM A MAP TITLED "ZONING LOCATION SURVEY PREPARED FOR OLD DRIFTWAY LLC", BY ALL SEASONS LAND SURVEYING, DATED MARCH 31, 2023.
- THE PROPOSED STRUCTURE IS A TOTAL SIX BEDROOM RESIDENCE DWELLING. THE REQUIRED EFFECTIVE LEACHING AREA FOR THIS HOUSE, WHICH IS BASED UPON AN OBSERVED PERCOLATION RATE OF 1"/10 MIN., IS 742.5 SF.
- THE PROPOSED LEACHING AREA CONSISTS OF 54 LINEAR FEET OF GEOMATRIX GST 6218, WHICH WILL PROVIDE 756.0 S.F. OF EFFECTIVE LEACHING AREA, (54 x 14.0 = 756.0).
- THE PROPOSED 1500 GALLON SEPTIC TANK SHALL CONFORM TO THE SPECIFICATIONS OUTLINED IN THE STATE OF CONNECTICUT TECHNICAL STANDARDS.
- THERE WILL BE NO WELL WITHIN 75 FEET OF THE PROPOSED SEPTIC SYSTEM.
- A BENCHMARK SHALL BE ESTABLISHED IN THE FIELD BY A SURVEYOR.
- ALL BERM MATERIAL SHALL BE FREE OF LARGE STONE, LOGS, OR OTHER DEBRIS THAT MAY CREATE LARGE VOIDS. IT SHALL CONSIST OF COMPACTED NATIVE LOAMY SOIL WITH A MAXIMUM PERCOLATION RATE OF 1"/15 MINUTES.
- ALL FILL SHALL BE PLACED ON THE PERIMETER OF THE PROPOSED LEACHING SYSTEM AND CAREFULLY PLACED BY THE CONTRACTOR IN LIFTS OF 1" MAXIMUM USING A SMALL CRAWLER, TRACTOR OR OTHER APPROVED MACHINERY.
- DISTRIBUTION BOXES SHALL BE ON STABLE FOOTING, CONSISTING OF 10" CRUSHED STONE.
- ALL LOCATIONS OF INLETS AND OUTLETS FROM THE SEPTIC TANK AND DISTRIBUTION BOXES SHALL BE GASKETED.
- ALL SELECT FILL MATERIAL MUST MEET THE REQUIREMENTS SPECIFIED IN SECTION VII A OF THE STATE OF CONNECTICUT PUBLIC HEALTH CODE TECHNICAL STANDARDS. A SIEVE ANALYSIS MUST BE SUBMITTED AND APPROVED BY THE ENGINEER PRIOR TO THE INSTALLATION.
- ALL SELECT FILL SHALL HAVE FEWER THAN 2.5% OF THE FINES PASSING THE #200 SIEVE, FEWER THAN 5% OF THE FINES PASSING #100 SIEVE, AND SHALL ACHIEVE AN AVERAGE PERCOLATION RATE OF 1"/5 MINUTES, AFTER BEING PLACED. ALL FILL SHALL BE PLACED IN LIFTS OF 1" MAXIMUM.
- THE CONTRACTOR SHALL PROVIDE A REPRESENTATIVE SAMPLE OF ALL FILL MATERIAL TO THE ENGINEER OF RECORD FOR INSPECTION AND SIEVE ANALYSIS AT THE CONTRACTOR'S EXPENSE, PRIOR TO PLACEMENT. IT MAY BE ALSO NECESSARY FOR THE INSTALLER TO PROVIDE A SAMPLE OF THE SOIL TO THE LOCAL HEALTH DEPARTMENT.
- THE SELECT FILL SHALL BE HARROWED INTO EXISTING SOIL, PAST THE TOPSOIL LAYER.
- A RESERVE AREA OF 42 L.F. OF GEOMATRIX GST 6224 IS SHOWN ON THE PLAN.
- THE SEPTIC TANK SHALL BE WATER-TIGHT AND BE SO CERTIFIED BY THE MANUFACTURER.
- THE CONTRACTOR SHALL MORTAR ALL INLETS AND OUTLETS FROM SEPTIC TANK AND PUMP CHAMBER ONCE PIPES HAVE BEEN INSTALLED.
- THE CONTRACTOR SHALL REMOVE ALL TREES, STUMPS, AND LARGE STONES WITHIN LIMITS OF THE SEWAGE DISPOSAL SYSTEM.
- THE CONTRACTOR SHALL STRIP AND STOCKPILE TOPSOIL OUTSIDE THE LIMITS OF SEWAGE DISPOSAL SYSTEM AND REUSE IT TO FINISH GRADE THE AREA OF DISTURBANCE, ADDITIONAL TOPSOIL, IF REQUIRED TO COVER DISTURBED AREAS, SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL TOPSOIL, FINE RAKE, SEED AND MULCH ALL AREAS DISTURBED BY CONSTRUCTION.
- WHERE POSSIBLE THE CONTRACTOR SHALL SAVE EXISTING TREES IN AND AROUND THE AREA OF THE PROPOSED SEWAGE DISPOSAL SYSTEM BY WHATEVER MEANS HE DEEMS PRUDENT. NO TREES ARE TO BE REMOVED WITHOUT THE AUTHORIZATION OF THE OWNER.
- THIS SYSTEM IS NOT DESIGNED TO ACCOMMODATE ANY FIXTURE FROM LEVELS LOWER THAN THE FIRST FLOOR. USE OF EJECTOR PUMP IS ASSUMED.
- ALL UTILITY LOCATIONS ARE APPROXIMATE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO CONFIRM THE LOCATION OF THE UTILITIES IN THE FIELD BY WHATEVER MEANS HE DEEMS PRUDENT.
- THE PUMP SHALL BE LIBERTY SERIES 290 OR EQUAL.
- THE PUMP SHALL BE EQUIPPED WITH A FLOAT SWITCH SET SO THAT THE PUMP DISCHARGES 150 GALLON PER CYCLE. ELECTRICAL HOOKUP TO THE ALARM SHALL BE PLACED IN A MINIMUM 4" BY 4" WEATHER TIGHT BOX SET A MINIMUM OF 12" ABOVE FINISHED GRADE IN A PROTECTED LOCATION. A SERVICE DISCONNECT SHALL BE PLACED WITHIN VIEW SERVICE DISCONNECT SHALL BE PLACED WITHIN VIEW OF THE PUMP CHAMBER.
- THE PUMP SHALL BE EQUIPPED WITH A WARNING SYSTEM THAT SHALL BE VISIBLE AND AUDIBLE TO THE HOMEOWNER.
- THE PUMP TESTING SHALL BE WITNESSED BY THE DESIGN ENGINEER AND THE WILTON HEALTH DEPARTMENT. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO INFORM THE ENGINEER AND THE HEALTH DEPARTMENT WHEN THE SYSTEM IS READY FOR TESTING.
- THE PROPOSED PUMP CHAMBER SHALL BE A 1000 GALLON SEPTIC TANK WITH ACCESS MANHOLE TO GRADE AS MANUFACTURED BY ARROW CONCRET PRODUCTS, INC. (CATALOG NO. ST1000), OR EQUAL. THE PROPOSED PUMP CHAMBER SHALL HAVE A MINIMUM RESERVE CAPACITY OF 600 GALLONS.
- THIS SYSTEM IS NOT DESIGNED TO ACCEPT WASTE FROM GARBAGE DISPOSAL UNITS, BACKWASH FROM WATER SOFTENER UNITS OR DISCHARGE FROM JACUZZI TYPE HOT TUBS (> 100 GALLONS).
- CONTRACTOR SHALL BE RESPONSIBLE FOR CALLING "CALL BEFORE YOU DIG", 1-800-922-4455, PRIOR TO START OF ANY EXCAVATION WORK ON SITE, TO LOCATE ALL UNDERGROUND UTILITIES ON PROPERTY AND SHOW SERVICE LINES TO BUILDING. EXCAVATIONS WITHIN 5 TO 25 FEET OF THE SEPTIC SYSTEM SHALL NOT BE BACKFILLED WITH FREE DRAINING MATERIAL.
- THIS DESIGN CONFORMS TO APPLICABLE CODES AND ACCEPTED PRACTICE, NO OTHER WARRANTY IS EXPRESSED OR IMPLIED.
- AN "AS BUILT" PLAN BY A SURVEYOR AND REVIEWED BY A PROFESSIONAL ENGINEER, SHALL BE SUBMITTED TO THE DEPARTMENT OF HEALTH BEFORE A "PERMIT TO USE" IS ISSUED.
- A CONNECTICUT REGISTERED PROFESSIONAL ENGINEER ACCEPTABLE TO THE DIRECTOR OF HEALTH SHALL REVIEW CONSTRUCTION TO INSURE COMPLIANCE WITH THE PROPOSED PLAN.
- THIS SYSTEM IS DESIGNED FOR A MAXIMUM DAILY WATER USE OF 675 GALLONS.
- MLSS REQUIREMENTS:
6 BEDROOM DWELLING:
FF = 2.25
PF = 1.0
RL = 45.0" AVG.; 54.0" U.G. AVG. (TH 6 & TH 10); 36.0" D.G. AVG. (TH 11)
SL = 6.7%
HF = 18
MLSS = (2.25) (1.0) (18) = 40.5 L.F.
SPREAD PROVIDED: 54 L.F.

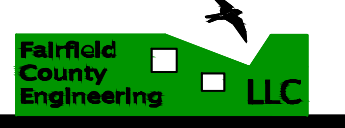


SEDIMENTATION AND EROSION CONTROL NOTES

- LAND DISTURBANCE SHALL BE KEPT TO A MINIMUM. PERMANENT STABILIZATION SHALL BE SCHEDULED AS SOON AS FINAL GRADES ARE ESTABLISHED.
- ALL DISTURBED AREAS SHALL BE FINE GRADED AND SEEDED WITH AN APPROVED SEED MIXTURE. COVER NEWLY SEEDED AREAS WITH MULCH HAY OR SALT HAY.
- ALL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE STANDARDS AND SPECIFICATIONS OF THE 2002 CONNECTICUT "GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL" HANDBOOK.
- ALL CONTROL MEASURES SHALL BE MAINTAINED IN EFFECTIVE CONDITION THROUGHOUT THE CONSTRUCTION PERIOD. CHECK AFTER EACH STORM EVENT.
- ADDITIONAL CONTROL MEASURES SHALL BE INSTALLED DURING THE CONSTRUCTION PERIOD, IF REQUIRED BY TOWN AUTHORITIES.
- SEDIMENT DEPOSITS REMOVED FROM FILTER BARRIERS SHALL BE PLACED IN FILL AREAS OR SPREAD WHERE THERE IS PROPOSED VEGETATIVE COVER. ANY SEDIMENT DEPOSITS REMAINING AFTER THE FILTER BARRIER IS REMOVED SHALL BE FINE GRADED AND PLANTED ACCORDING TO PLAN.
- THE SITE CONSTRUCTION CONTRACTOR IS ASSIGNED THE RESPONSIBILITY FOR IMPLEMENTING THIS EROSION AND SEDIMENT CONTROL PLAN. THIS RESPONSIBILITY INCLUDES THE INSTALLATION AND MAINTENANCE OF CONTROL MEASURES, INFORMING ALL PARTIES ENGAGED ON THE CONSTRUCTION SITE OF THE REQUIREMENTS AND OBJECTIVES OF THE PLAN, NOTIFYING THE PLANNING AND ZONING OFFICE (AND/OR THE CONSERVATION COMMISSION) OF ANY TRANSFER OF THIS RESPONSIBILITY AND CONVEYING A COPY OF THE EROSION AND SEDIMENT CONTROL PLAN IF THE TITLE TO THE LAND IS TRANSFERRED TO A NEW OWNER.



7-26-23
date



REV. 2/19/24: SIX BEDROOMS. REV. 1/15/24: TEST HOLES REV. 12/16/23: PER WETLANDS.	
OLD DRIFTWAY LLC 0 MOUNTAIN ROAD WILTON, CONNECTICUT	
SEPTIC PLAN	
CIVIL ENGINEERS	2168 project
FAIRFIELD COUNTY ENGINEERING L.L.C. 60 WINFIELD STREET, NORWALK, CONNECTICUT 06855 PH: (203) 831-8005 FAX: (203) 831-8006	
1 OF 2 sheet	