#### **ENGINEERING REPORT**

#### STORMWATER MANAGEMENT PLAN 3 HUBBARD ROAD WILTON, CONNECTICUT

January 20, 2020

#### **Prepared for:**

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#### STORMWATER MANAGEMENT PLAN 3 Hubbard Road Wilton, CT

#### **PROJECT DESCRIPTION**

The descriptions and computations included within this Stormwater Management Plan and Appendix are provided in support of applications submitted by 3 Hubbard Road, LLC to the various regulatory agencies for permitting purposes. Since the property is tied into the Town's street drainage system, and there are changes in coverage of the existing property, the site's Storm water runoff system shall be sized to accommodate runoff from a 25-year design storm and the system shall be designed so as to prevent runoff from parking lots, roofs, and access drives from flowing into the street. Per Town requirements, the storm water management plan has been prepared by a Connecticut-licensed professional engineer.

#### Location

The project site is located at 3 Hubbard Road, Wilton, Connecticut (site); As shown in Figure 1 the site is located in the Wilton Center District. The 1.27-acre site is a fully developed urban area with vegetation established.



Figure 1 Project Location - 3 Hubbard Road, Wilton, CT.

#### **Existing Conditions**

The site has an existing apartment building built in 1943. The impervious area created by the existing building is 6,267 square feet. Other impervious areas in the property include driveways and parking areas, a patio, and access sidewalks. The site's open space are landscaped grass areas, and a woods area with slopes over 30%. The purpose of the proposed project is to build an additional three story high residential building with 17 units. The new building will be located in the existing woods area of the property and there will not be any other permanent modification to pervious or impervious areas on the site.

There are three separate watersheds (WS) in the property. As shown on Appendix A, WS-1 covers most of the rear of the property, West side. WS-1 includes a portion of the existing building, a patio, grass areas, driveways, parking areas, and wood areas; there is an existing on-site stormwater drain system that discharges to the Town's drain system on Old Ridgefield Road. WS-2 is a smaller area also in the back of the property and it is all wood areas. The front of the property is WS-3 and there are no proposed permanent modification or changes to the pervious or impervious areas in this watershed. WS-3 drains East to the Town's street drainage system on Hubbard Road.

Based upon FEMA's Flood Insurance Rate Map (FIRM) Map number 09001C0383F for Fairfield County, effective date June 18, 2010, the site is located entirely within Zone X, an area determined to be outside the 0.2% annual chance floodplain (See Appendix C).

According to the Soil Survey of Fairfield County, the site's soil type where the building will be Located and where the storm water management infiltration system will be installed is entirely Charlton-Chatfield complex, 15 to 45% slopes, very rocky land complex with a "well drained" natural drainage classification. Charlton soils belong to soil group B (See Appendix D). There are no wetlands or surface water bodies on site. Groundwater in the area is classified with a GA ground water quality class. The water quality classes are based on the "Water Quality Classification Norwalk, CT" map prepared by CT DEEP dated November 2015 (See Appendix E).

#### **Proposed Project**

The proposed stormwater management system for this project is designed to collect and direct runoff to a subsurface infiltration system. The system will provide infiltration, temporary storage of runoff, reduction of post development peak discharge rates, and meet groundwater recharge volume and pollutant reduction requirements. The system has been designed in accordance with the Town of Wilton Zoning Regulations, and the guidelines and procedures set forth in the 2004 Connecticut Storm Quality Manual.

The proposed development will impact WS-1 and WS-2 (Appendix B). The required grading and new building will modify the size of the watersheds and will increase the size of the impervious areas. The proposed WS-1 is 80% impervious (paved parking lot, roofs, etc.). The proposed stormwater management system includes a 12" storm pipe overflow that connects to the existing 12" storm drain in the parking lot. The existing system discharges on the North side of the property into the municipal storm sewer network. The proposed and existing 12" drains are sized to convey the peak runoff discharge of the 25-year storm. Details of the size and location of the stormwater management system can be found on drawing GD-1 of the design plans.

Based on our experience, soils group B throughout the Town of Wilton are very well drained with a typical percolation rate of 5-10 mpi (minutes per inch). Owner shall perform a percolation test during construction to verify site conditions.

The system has been designed to provide infiltration of the first inch of runoff, removing at least 80% of the average annual total suspended solids (TSS) load, removing at least 80% of floatable debris, providing peak runoff rate attenuation, and volume attenuation to improve the existing site hydrology, as well as to minimize sediment transport.

The area of the proposed WS-2 is about 50% smaller than the WS-2 area under existing conditions.

#### PEAK RUNOFF CALCULATIONS

Calculations for the existing and proposed conditions were analyzed using the US Department of Agriculture Natural Resources Conservation Service (NRCS, Formerly known as the Soil Conservation Service or SCS) TR-55 methodology which outlines procedures for calculating storm runoff volume, peak rate of discharge, and hydrographs on small watersheds. To complement the design results, the method utilized to predict the surface water runoff rate in this analysis and the impact of the proposed infiltration/detention system is a computer program entitled Hydraflow Hydrographs Extension for AutoCAD Civil 3D 2020. The Hydrographs computer modeling program utilizes the same methods for computing runoff rates that were originally developed by the US Department of Agriculture NRCS, and they are also utilized in the TR-20 computer modeling program and others. Calculations are included in Appendix F.

The runoff curve number is a land sensitive coefficient that dictates the relationship between total rainfall depth and direct storm runoff. The site as stated before is mostly fully developed urban area with paved parking lots, roofs, and driveways.

The time of concentration (Tc) is defined as the time for runoff to travel from the hydraulically most distant point in the watershed to a point of interest. Values of time of concentration were determined for both conditions based on land cover and slope of the flow path using methods outlined in TR-55. Considering the steep slopes, a time of concentration of 0.1 hours was used.

For this study, all rainfall depth and rainfall intensity were obtained from the National Oceanic and Atmospheric Administration (NOAA) Weather Service Hydrometeorological Design Studies Center Precipitation Frequency Data Server (PDFS). Enclosed for reference is a copy of the downloaded NOAA Atlas 14 table for rainfall depth and rainfall intensity (See Appendix G).

The peak flow and total runoff volume were determined for all frequency storms analyzed. The total runoff volumes, and peak flows are summarized below:

Storm Frequency	Existing Conditions Runoff Volume (cf)	Proposed Conditions Runoff Volume (cf)	% decrease
1-Year	4471	3300	-26%
2-Year	5810	4800	-17%
5-Year	8087	7383	-9%
10-Year	9973	9543	-4%
25-Year	12627	12600	-0.21%

Table 1 Site Runoff Volume (cf) Comparison for WS-1

#### Storm Frequency **Existing Conditions Proposed Conditions** % decrease Peak Discharge (cfs) Peak Discharge (cfs) 1.94 -38% 1-Year 1.21 2-Year 2.53 2.04 -19% 5-Year 3.52 2.54 -28% 10-Year 4.34 3.18 -27% 25-Year 5.49 4.06 -26%

Table 2 Site Peak Discharge (cfs) Comparison for WS-1

Table 3 Site Runoff Volume (cf) Comparison for WS-2

Storm Frequency	Existing Conditions	Proposed Conditions	% decrease
	Runoff Volume (cf)	Runoff Volume (cf)	
1-Year	671	141	-79%
2-Year	247999	247	-75%
5-Year	1611	467	-71%
10-Year	2154	677	-69%
25-Year	2953	1003	-66%

Table 4 Site Peak Discharge (cfs) Comparison for WS-2

Storm Frequency	Existing Conditions	Proposed Conditions	% decrease
	Peak Discharge (cfs)	Peak Discharge (cfs)	
1-Year	0.28	0.04	-84%
2-Year	0.42	0.09	-78%
5-Year	0.68	0.19	-72%
10-Year	0.92	0.28	-70%
25-Year	1.28	0.42	-67%

As shown on the tables above, the reduction of the peak runoff rate and runoff volume off-site in WS-1 is achieved through the implementation of the underground infiltration/detention system which provides temporary storage for runoff. The proposed retention system will be surrounded by 12 inches of stone to provide a stable base and to increase the storage volume.

As stated earlier the size of WS-2 will be decreased by 50%, and replacing wood areas for grass landscaped areas also contributes to the reduction of the peak runoff rate and runoff volume off-site in WS-2.

The underground infiltration/detention system will be located under the parking lot area, and the system capacity is designed to also capture the stormwater runoff generated by the first inch of all rainfall events. The volume of the infiltration system meets the requirements of the groundwater recharge criteria and the pollutant reduction criteria set on the 2004 CTDEEP Stormwater Quality Manual.

Included as Appendix H is a copy of the "Maintenance & Inspection Schedule" which has been prepared for the site to promote good housekeeping measures.

Design plans accompany this report to provide additional design detail regarding the underground storage system proposed.

APPENDIX A





Approximate Scale: 1 inch = 100 feet

Appendix A. Project Location and Existing Watershed Areas

Map Grand List Date: Oct 2017

0 50 100 150

APPENDIX B



APPENDIX C

## National Flood Hazard Layer FIRMette



#### Legend



APPENDIX D



United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for State of Connecticut

3 Hubbard Rd, Wilton CT



### Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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### **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

#### Custom Soil Resource Report Soil Map



	MAP L	EGEND		MAP INFORMATION
Area of Int	erest (AOI)	3	Spoil Area	The soil surveys that comprise your AOI were mapped at
	Area of Interest (AOI)	٥	Stony Spot	1:12,000.
Soils		۵	Very Stony Spot	Warning: Soil Map may not be valid at this scale
	Soil Map Unit Polygons	Ŷ	Wet Spot	
$\sim$	Soil Map Unit Lines	Δ	Other	Enlargement of maps beyond the scale of mapping can cause
	Soil Map Unit Points		Special Line Features	line placement. The maps do not show the small areas of
Special	Point Features	Water Fea	tures	contrasting soils that could have been shown at a more detailed
<u>ه</u>	Biowoul	~	Streams and Canals	
X	Borrow Pit	Transport	ation	Please rely on the bar scale on each map sheet for map
×	Clay Spot	+++	Rails	measurements.
$\diamond$	Closed Depression	~	Interstate Highways	Source of Map: Natural Resources Conservation Service
X	Gravel Pit	~	US Routes	Web Soil Survey URL:
0 0 0	Gravelly Spot	$\sim$	Major Roads	Coordinate System: Web Mercator (EPSG:3857)
0	Landfill	~	Local Roads	Maps from the Web Soil Survey are based on the Web Mercator
A.	Lava Flow	Backgrou	nd	projection, which preserves direction and shape but distorts
عليه	Marsh or swamp	No.	Aerial Photography	Albers equal-area conic projection, should be used if more
R	Mine or Quarry			accurate calculations of distance or area are required.
0	Miscellaneous Water			This product is generated from the USDA-NRCS certified data as
0	Perennial Water			of the version date(s) listed below.
$\vee$	Rock Outcrop			Soil Survey Area: State of Connecticut
+	Saline Spot			Survey Area Data: Version 19, Sep 13, 2019
	Sandy Spot			Soil man units are labeled (as snace allows) for man scales
-	Severely Eroded Spot			1:50,000 or larger.
~	Sinkhole			Deta/a) social images were photographed. Dec 21, 2000. Opt 5
2	Slide or Slip			2016
» B	Sodic Spot			
jø				I ne orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

### **Map Unit Legend**

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
60D	Canton and Charlton soils, 15 to 25 percent slopes	1.4	43.8%
73E	Charlton-Chatfield complex, 15 to 45 percent slopes, very rocky	0.7	22.2%
307	Urban land	1.1	34.0%
Totals for Area of Interest		3.2	100.0%

### **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

#### State of Connecticut

#### 60D—Canton and Charlton soils, 15 to 25 percent slopes

#### **Map Unit Setting**

National map unit symbol: 9lpq Elevation: 0 to 1,200 feet Mean annual precipitation: 43 to 54 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 185 days Farmland classification: Not prime farmland

#### **Map Unit Composition**

Canton and similar soils: 45 percent Charlton and similar soils: 35 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Canton**

#### Setting

Landform: Hills Down-slope shape: Linear Across-slope shape: Convex Parent material: Coarse-loamy over sandy and gravelly melt-out till derived from granite and/or schist and/or gneiss

#### **Typical profile**

*Oe - 0 to 1 inches:* moderately decomposed plant material *A - 1 to 3 inches:* gravelly fine sandy loam *Bw1 - 3 to 15 inches:* gravelly loam *Bw2 - 15 to 24 inches:* gravelly loam *Bw3 - 24 to 30 inches:* gravelly loam *2C - 30 to 60 inches:* very gravelly loamy sand

#### **Properties and qualities**

Slope: 15 to 25 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 5.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4e Hydrologic Soil Group: B Hydric soil rating: No

#### **Description of Charlton**

#### Setting

Landform: Hills Down-slope shape: Linear Across-slope shape: Linear Parent material: Coarse-loamy melt-out till derived from granite and/or schist and/or gneiss

#### **Typical profile**

Ap - 0 to 4 inches: fine sandy loam Bw1 - 4 to 7 inches: fine sandy loam Bw2 - 7 to 19 inches: fine sandy loam Bw3 - 19 to 27 inches: gravelly fine sandy loam C - 27 to 65 inches: gravelly fine sandy loam

#### **Properties and qualities**

Slope: 15 to 25 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 5.9 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 4e Hydrologic Soil Group: B Hydric soil rating: No

#### Minor Components

#### Leicester

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

#### Chatfield

Percent of map unit: 5 percent Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

#### Sutton

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

#### Hollis

Percent of map unit: 5 percent Landform: Hills, ridges Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

#### 73E—Charlton-Chatfield complex, 15 to 45 percent slopes, very rocky

#### Map Unit Setting

National map unit symbol: 9lql Elevation: 0 to 1,200 feet Mean annual precipitation: 43 to 56 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 140 to 185 days Farmland classification: Not prime farmland

#### **Map Unit Composition**

Charlton and similar soils: 45 percent Chatfield and similar soils: 30 percent Minor components: 25 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Charlton**

#### Setting

Landform: Hills Down-slope shape: Linear Across-slope shape: Linear Parent material: Coarse-loamy melt-out till derived from granite and/or schist and/or gneiss

#### **Typical profile**

Ap - 0 to 4 inches: fine sandy loam Bw1 - 4 to 7 inches: fine sandy loam Bw2 - 7 to 19 inches: fine sandy loam Bw3 - 19 to 27 inches: gravelly fine sandy loam C - 27 to 65 inches: gravelly fine sandy loam

#### Properties and qualities

Slope: 15 to 45 percent
Percent of area covered with surface fragments: 1.6 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None

*Frequency of ponding:* None *Available water storage in profile:* Low (about 5.9 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B Hydric soil rating: No

#### **Description of Chatfield**

#### Setting

Landform: Ridges, hills Down-slope shape: Convex Across-slope shape: Linear Parent material: Coarse-loamy melt-out till derived from granite and/or schist and/or gneiss

#### **Typical profile**

*Oa - 0 to 1 inches:* highly decomposed plant material *A - 1 to 6 inches:* gravelly fine sandy loam *Bw1 - 6 to 15 inches:* gravelly fine sandy loam *Bw2 - 15 to 29 inches:* gravelly fine sandy loam *2R - 29 to 80 inches:* unweathered bedrock

#### **Properties and qualities**

Slope: 15 to 45 percent
Percent of area covered with surface fragments: 1.6 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Natural drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Low to high (0.01 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Low (about 3.3 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: B Hydric soil rating: No

#### **Minor Components**

#### Rock outcrop

*Percent of map unit:* 10 percent *Hydric soil rating:* No

#### Leicester

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Linear Across-slope shape: Concave Hydric soil rating: Yes

#### Sutton

Percent of map unit: 5 percent Landform: Drainageways, depressions Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

#### Hollis

Percent of map unit: 3 percent Landform: Hills, ridges Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

Unnamed, sandy subsoil Percent of map unit: 1 percent Hydric soil rating: No

#### Unnamed, red parent material

Percent of map unit: 1 percent Hydric soil rating: No

#### 307—Urban land

#### Map Unit Setting

National map unit symbol: 9lmh Elevation: 0 to 2,000 feet Mean annual precipitation: 43 to 56 inches Mean annual air temperature: 45 to 55 degrees F Frost-free period: 120 to 185 days Farmland classification: Not prime farmland

#### Map Unit Composition

*Urban land:* 80 percent *Minor components:* 20 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Urban Land**

#### Typical profile

H - 0 to 6 inches: material

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 8 Hydrologic Soil Group: D Hydric soil rating: Unranked

#### **Minor Components**

#### Unnamed, undisturbed soils

*Percent of map unit:* 10 percent *Hydric soil rating:* No

#### Udorthents, wet substratum

Percent of map unit: 10 percent Down-slope shape: Convex Across-slope shape: Linear Hydric soil rating: No

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APPENDIX E

# WATER QUALITY CLASSIFICATIONS NORWALK, CT

# SURFACE WATER QUALITY CLASSES



Surface Water Classifications beginning with S refer to Coastal and Marine Surface Water. B\* is a subset of Class B where no direct wastewater discharges are allowed other than those consistent with Class AA, A and SA surface waters.

# **GROUND WATER QUALITY CLASSES**

GA (white background)

GAA, GAAs

GA, GAA may not meet current standards GB GC

> Final Aquifer Protection Area (Level A) — — • Major Basin Boundary

# EXPLANATION

WATER QUALITY CLASSIFICATIONS (WQC) MAPS are one of the elements of the Water Quality Standards (WQS) for the State of Connecticut. The WQS are a part of Connecticut's clean water program and are essential for protecting and improving water quality. The WQS follow the principles of Connecticut's Clean Water Act which is in Chapter 446K of the Connecticut General Statutes. The WQS provide policy guidance in many areas, for example decisions on acceptable discharges to water resources, siting of landfills, remediation or prioritization of municipal sewerage system projects. The first two elements of the WQS are the Standards, which set an overall policy for management of water quality, and the Criteria, which are descriptive and numerical standards that describe the allowable parameters and goals for various water quality classifications. A discussion of these two elements is found in the Water Quality Standards document available on the CT DEEP website. The third element is the Classifications and the Water Quality Classification Maps which show the Classification assigned to each surface and groundwater resource throughout the State. The WQS are adopted using a public participation process. The WQC maps are also adopted using a public participation process but go through hearings separately from the Standards and Criteria hearings. Revision and adoption of the WQC data occurs in accordance with the public participation procedures contained in Section 22a-426 of the Connecticut General Statutes. Ground WQC is subject to Connecticut regulation and changes must be reviewed and adopted. All changes to the Surface WQC require an adoption process which is subject to federal review and approval in addition to CT regulation. The adoption dates for the WQC by major drainage basin are: Housatonic River, Hudson River and Southwest Coastal Basins -March 1999; Connecticut River and South Central Coastal Basins -February 1993; Thames River, Pawcatuck River and Southeast Coastal Basins - December 1986. Surface Water Classifications do not change after the adoption date until the next major revision. Ground Water Classifications may change after the adoption date under specific circumstances. The map may have more than one WQC adoption date because a town may be in more than one major drainage basin.

SURFACE WATERS in Connecticut are divided into freshwater classified as AA, A, B or B\* and saline waters classified as SA or SB. Class AA designated uses are existing or proposed drinking water supplies; habitat for fish and other aquatic life and wildlife; recreation; and water supply for industry and agriculture. Class A designated uses are habitat for fish and other aquatic life and wildlife; potential drinking water supplies; recreation; navigation; and water supply for industry and agriculture. Class SA designated uses are habitat for marine fish, other aquatic life and wildlife; shellfish harvesting for direct human consumption; recreation; industrial water supply; and navigation. Class B designated uses are habitat for fish and aquatic life and wildlife; recreation; navigation; and industrial and agricultural water supply. Class B\*, applicable to Candlewood Lake, is a subset of Class B and is identical in all ways to the designated uses, criteria and standards for Class B waters except for the restriction on direct discharges. Class SB designated uses are habitat for marine fish and aquatic life and wildlife; commercial shellfish harvesting; recreation;

industrial water supply; and navigation.

Surface waters which are not specifically classified shall be considered as Class A or Class AA. Surface waters in GA ground water areas are assumed Class A or Class SA unless otherwise indicated. Surface waters in GAA ground water areas are assumed Class AA unless otherwise indicated.

Area of Contribution to Public Supply Well

On the WQC map a surface water quality goal of A is represented by blue colored water bodies. Surface water quality goal of AA is represented by purple colored water bodies. Surface water quality goal of B is represented by gold colored water bodies.

GROUND WATERS in Connecticut are classified as GAA, GA, GB and GC. Class GAA designated uses are existing or potential public supply of water suitable for drinking without treatment and baseflow for hydraulically-connected surface water bodies. The Class GAAs is a subclass of GAA for ground water that is tributary to a public water supply reservoir. The area of contribution to a public water supply well is represented by a 500-foot radius around the well and is assumed to be Class GAA unless otherwise classified. Class GA designated uses are existing private and potential public or private supplies of water suitable for drinking without treatment and baseflow for hydraulically-connected surface water bodies. All ground waters not specifically classified are considered as Class GA. Class GB designated uses are industrial process water and cooling waters and baseflow for hydraulically-connected water bodies and is presumed not suitable for human consumption without treatment. Class GC designated uses are assimilation of discharges authorized by the Commissioner pursuant to Section 22a-430 of the General Statutes.

On the WQC map GA is represented by white colored land areas. Class GAA and class GAAs are represented by blue colored land areas. The area of contribution to a public water supply well is shown by a blue cross-hatch overprint. A notation of GAA followed by a state abbreviation indicates a watershed that contributes to the public water supply for a state other than Connecticut. Class GA or Class GAA areas that currently may not be meeting the GA or GAA standards are represented on the WQC maps by tan colored land areas. Class GB is represented by green colored land areas. Class GC is represented by magenta colored land areas.

FINAL AQUIFER PROTECTION AREAS (Level A) are included on the WQC maps for informational purposes. These areas are anticipated to be reclassified GAA during the next major basin updates, subject to public participation. The Aquifer Protection Program helps protect Connecticut's public drinking water resources by delineating aquifer protection areas (also called wellhead protection areas) for public supply wells and establishing land use regulations within these areas. These areas represent the land area contributing ground water to active public water supply wells or well fields that serve more than 1000 people and are set in sand and gravel aquifers (stratified drift deposits).

# DATA SOURCES

WATER QUALITY CLASSIFICATIONS DATA – Water quality classifications shown on this map are based on information from the following digital spatial datasets that are typically shown together – Ground Water Quality Classifications Poly, Surface Water Quality Classifications Line, and Surface Water Quality Classifications Poly. The map legend above reflects the content of these three data sources. These WQC data were initially compiled on 1:24,000-scale 7.5 minute USGS topographic quadrangle maps and later digitized at 1:24,000 scale. For example, the Surface Water Quality Classifications Line and Surface Water Quality Classifications Poly digital data assigns surface water quality classifications to water bodies such as rivers, streams, reservoirs, lakes, ponds and coves found in 1:24,000-scale hydrography data available from CT DEEP. The hydrography may not include all the waterbodies in Connecticut. The Ground Water Quality Classifications Poly data assigns ground water quality classifications, at 1:24,000 scale, to the remaining land areas in Connecticut.

AQUIFER PROTECTION AREA DATA – Aquifer Protection Areas shown on this map are from the Aquifer Protection Area digital dataset which contains polygon data intended to be used at 1:24,000 scale. The dataset contains regulated areas classified as Level A Aquifer Protection Area (Final) and Level B Aquifer Protection Area (Preliminary). The Level B areas are not shown on the WQC maps. The data was collected from 1991 to the present and is actively updated as Final area mapping replaces earlier Preliminary areas. The Aquifer Protection Areas are delineated by

### **ADOPTED DATES**

Water Quality Standards February 25, 2011

Thames River, Pawcatuck River and Southeast Coastal Basins: December 1986

Connecticut River and South Central Coastal Basins: February 1993

Housatonic River, Hudson River and Southwest Coastal Basins: March 1999

submitted to the CT DEEP for approval. Preliminary mapping provides a general estimate of the area contributing ground water to the well field. Final mapping is based on extensive, site-specific, detailed modeling of the ground water flow system. CT DEEP may adjust Final area boundaries to be consistent with 1:24,000 scale topography and base map data where appropriate during the

MAJOR DRAINAGE BASIN DATA - Major drainage basins shown on this map are from Major Basin Line data developed by

BASE MAP DATA - Based on data originally from 1:24,000-scale USGS 7.5 minute topographic quadrangle maps published between 1969 and 1992. It includes political boundaries, railroads, airports, hydrography, geographic names and geographic places. Streets and street names are from Tele Atlas<sup>®</sup> copyrighted data. Base map information is neither current nor complete.

WATER QUALITY STANDARDS - Go to the CT DEEP website for a summary and the full text of the "Water Quality Standards" AQUIFER PROTECTION AREAS - Go to the CT DEEP website



0.5 0 SCALE 1:24,000 (1 inch = 2000 feet) when map is printed at original size



STATE OF CONNECTICUT DEPARTMENT OF ENERGY & ENVIRONMENTAL PROTECTION 79 Elm Street Hartford, CT 06106-5127

Map created by CT DEEP October 2017 Map is not colorfast Protect from light and moisture

the individual water companies owning the well fields and

approval process.

CT DEEP and intended to be used at 1:24,000 scale.

**RELATED INFORMATION** 

This map is intended to be printed at its original dimensions in order to maintain the 1:24,000 scale (1 inch = 2000 feet). and for other information on water quality. for more information.



APPENDIX F

### TR 55 PEAK RUNOFF CALCULATION (GRAPHICAL METHOD)

ver 12-2010

CLIENT:	3 HUBBARD RD	CITY	WILTON	DATE:	1/16/2020
DSN BY:	RCEBALLOS	CHK BY:		DATE:	
COMMENTS:	<b>Existing Conditions - WS-</b>	1			

Drainage Area	0.64156	Acres
Runoff Curve Number	90	

Time of Concentration

0.100 Hours

Frequency	yr	1	2	5	10	25	50	100
Rainfall, P (24 hour)	in	2.93	3.55	4.58	5.42	6.59	7.46	8.39
Initial Abstraction, la	in	0.222	0.222	0.222	0.222	0.222	0.222	0.222
la/P ratio		0.076	0.063	0.048	0.041	0.034	0.030	0.026
Unit Peak Discharge, qu	cfs/ac/in	1.578	1.578	1.578	1.578	1.578	1.578	1.578
Runoff	in	1.92	2.50	3.47	4.28	5.42	6.27	7.19
Runoff Volume	ac-ft	0.103	0.133	0.186	0.229	0.290	0.335	0.384
Peak Discharge, qp	cfs	1.94	2.53	3.52	4.34	5.49	6.35	7.28
Runoff Volume	cf	4471.76	5810.51	8087.26	9973.52	12627.00	14613.05	16744.38
# **GROUNDWATER RECHARGE CALCULATIONS**

CLIENT:	3 HUBBARD RD	CITY	WILTON		0	DATE:	1/16/2020	
DSN BY:	RCEBALLOS	CHK BY:			0	DATE:		
COMMENTS:	<b>Existing Condition</b>	ns - WS-1						
Impervious D Hydrologic Sc	rainage Area	17654 sf B						
HYDROLOGI	C GROUP VOLUME	TO RECHARGE (	x TOTAL IMPERVIC	OUS AREA)				
Hydrologic Gr	roup Vol	ume to Recharge x	Total Impervious A	rea				
А		0.6	6 inches of runoff					
В		0.35	5 inches of runoff					
С		0.25	5 inches of runoff					
D		0.2	1 inches of runoff					
Volume of Re	charge 51	4.9083 cf						

## STORMWATER INFILTRATION SYSTEM SIZING

CLIENT:	3 HUBBARD RD	CITY	WILTON	0	DATE:	1/16/2020
DSN BY:	RCEBALLOS	CHK BY:		0	DATE:	
COMMENTS:	<b>Existing Conditions - WS-</b>	1				

Sizing of the proposed stormwater infiltration system will be based on the stormwater runoff volume generated by the 1-inch rainfall event and the stormwater runoff generated by the first inch of all larger rainfall events. The sizing methodology meets the City's runoff reduction and water quality protection

Frequency	yr	ALL
Rainfall, P	in	1
Initial Abstraction, la	in	0.222
Ia/P ratio		0.222
Unit Peak Discharge, qu	cfs/ac/in	1.58
Runoff	in	0.32
Runoff Volume	ac-ft	0.02
Peak Discharge, qp	cfs	0.32
Infiltrator VOLUME	cf	746.2

## TR 55 PEAK RUNOFF CALCULATION (GRAPHICAL METHOD)

ver 12-2010

CLIENT:	3 HUBBARD RD	CITY	WILTON	DATE:	1/16/2020
DSN BY:	RCEBALLOS	CHK BY:		DATE:	
COMMENTS:	<b>Existing Conditions - WS-</b>	2			

Drainage Area	0.227	Acres
Runoff Curve Number	73	

Time of Concentration

0.100 Hours

Frequency	yr	1	2	5	10	25	50	100
Rainfall, P (24 hour)	in	2.93	3.55	4.58	5.42	6.59	7.46	8.39
Initial Abstraction, la	in	0.74	0.74	0.74	0.74	0.74	0.74	0.74
la/P ratio		0.253	0.208	0.162	0.137	0.112	0.099	0.088
Unit Peak Discharge, qu	cfs/ac/in	1.489	1.514	1.542	1.556	1.571	1.578	1.578
Runoff	in	0.81	1.21	1.96	2.61	3.58	4.33	5.16
Runoff Volume	ac-ft	0.015	0.023	0.037	0.049	0.068	0.082	0.098
Peak Discharge, qp	cfs	0.28	0.42	0.68	0.92	1.28	1.55	1.85
Runoff Volume	cf	671.13	999.67	1611.77	2154.03	2953.27	3571.58	4249.25

## TR 55 PEAK RUNOFF CALCULATION (GRAPHICAL METHOD)

ver 12-2010

CLIENT:	3 HUBBARD RD	CITY	WILTON	DATE:	1/16/2020
DSN BY:	RCEBALLOS	CHK BY:		DATE:	
COMMENTS:	<b>Proposed Conditions - WS</b>	6-1			

Drainage Area	0.7523	Acres
Runoff Curve Number	87	

Time of Concentration

0.100 Hours

Frequency	yr	1	2	5	10	25	50	100
Rainfall, P (24 hour)	in	2.93	3.55	4.58	5.42	6.59	7.46	8.39
Initial Abstraction, la	in	0.299	0.299	0.299	0.299	0.299	0.299	0.299
la/P ratio		0.102	0.084	0.065	0.055	0.045	0.040	0.036
Unit Peak Discharge, qu	cfs/ac/in	1.577	1.578	1.578	1.578	1.578	1.578	1.578
Runoff	in	1.68	2.23	3.17	3.96	5.08	5.92	6.83
Runoff Volume	ac-ft	0.105	0.140	0.199	0.249	0.319	0.371	0.428
Peak Discharge, qp	cfs	1.99	2.64	3.77	4.71	6.04	7.03	8.11
Runoff Volume	cf	4582.36	6082.36	8665.97	10825.82	13882.39	16179.51	18650.85

# **GROUNDWATER RECHARGE CALCULATIONS**

CLIENT:	<b>3 HUBBARD R</b>	D	CITY	WILTON		0	DATE:	1/16/2020	
DSN BY:	RCEBALLOS		CHK BY:			0	DATE:		
COMMENTS	Proposed Con	ditions - W	S-1						
Impervious D Hydrologic So	rainage Area bil Group	26558 B	sf						
HYDROLOGI	C GROUP VOLU	JME TO RE	CHARGE (	X TOTAL IMPERVIO	US AREA)				
Hydrologic G	roup	Volume to F	Recharge x	Total Impervious Are	ea				
А			0.6	inches of runoff					
В			0.35	inches of runoff					
С			0.25	inches of runoff					
D			0.1	inches of runoff					
Volume of Re	echarge	774.6083	cf						

## STORMWATER INFILTRATION SYSTEM SIZING

CLIENT:	3 HUBBARD RD	CITY	WILTON	0	DATE:	1/16/2020
DSN BY:	RCEBALLOS	CHK BY:		0	DATE:	
COMMENTS:	Proposed Conditions - WS	6-1				

Sizing of the proposed stormwater infiltration system will be based on the stormwater runoff volume generated by the 1-inch rainfall event and the stormwater runoff generated by the first inch of all larger rainfall events. The sizing methodology meets the City's runoff reduction and water quality protection

Frequency	yr	ALL
Rainfall, P	in	1
Initial Abstraction, la	in	0.299
la/P ratio		0.299
Unit Peak Discharge, qu	cfs/ac/in	1.58
Runoff	in	0.22
Runoff Volume	ac-ft	0.01
Peak Discharge, qp	cfs	0.27
Infiltrator VOLUME	cf	611.3

## TR 55 PEAK RUNOFF CALCULATION (GRAPHICAL METHOD)

ver 12-2010

CLIENT:	3 HUBBARD RD	CITY	WILTON	DATE:	1/16/2020
DSN BY:	RCEBALLOS	CHK BY:		DATE:	
COMMENTS:	<b>Proposed Conditions - WS</b>	6-2			

Drainage Area	0.1147	Acres
Runoff Curve Number	61	

Time of Concentration

0.100 Hours

Frequency	yr	1	2	5	10	25	50	100
Rainfall, P (24 hour)	in	2.93	3.55	4.58	5.42	6.59	7.46	8.39
Initial Abstraction, la	in	1.279	1.279	1.279	1.279	1.279	1.279	1.279
la/P ratio		0.437	0.360	0.279	0.236	0.194	0.171	0.152
Unit Peak Discharge, qu	cfs/ac/in	1.126	1.356	1.474	1.499	1.523	1.536	1.547
Runoff	in	0.34	0.60	1.12	1.63	2.41	3.04	3.74
Runoff Volume	ac-ft	0.003	0.006	0.011	0.016	0.023	0.029	0.036
Peak Discharge, qp	cfs	0.04	0.09	0.19	0.28	0.42	0.54	0.66
Runoff Volume	cf	141.08	247.84	467.99	677.75	1003.39	1265.02	1559.03

# Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

### Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 2.986 cfs
Storm frequency	= 10 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 9,349 cuft
Drainage area	= 0.641 ac	Curve number	= 90
Basin Slope	= 9.1 %	Hydraulic length	= 375 ft
Tc method	= LAG	Time of conc. (Tc)	= 3.39 min
Total precip.	= 5.42 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

### Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 1.389 cfs
Storm frequency	= 1 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 4,191 cuft
Drainage area	= 0.641 ac	Curve number	= 90
Basin Slope	= 9.1 %	Hydraulic length	= 375 ft
Tc method	= LAG	Time of conc. (Tc)	= 3.39 min
Total precip.	= 2.93 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

### Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 3.729 cfs
Storm frequency	= 25 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 11,836 cuft
Drainage area	= 0.641 ac	Curve number	= 90
Basin Slope	= 9.1 %	Hydraulic length	= 375 ft
Tc method	= LAG	Time of conc. (Tc)	= 3.39 min
Total precip.	= 6.59 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

### Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 1.787 cfs
Storm frequency	= 2 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 5,446 cuft
Drainage area	= 0.641 ac	Curve number	= 90
Basin Slope	= 9.1 %	Hydraulic length	= 375 ft
Tc method	= LAG	Time of conc. (Tc)	= 3.39 min
Total precip.	= 3.55 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

### Hyd. No. 2

Hydrograph type	= SCS Runoff	Peak discharge	= 2.449 cfs
Storm frequency	= 5 yrs	Time to peak	= 12.07 hrs
Time interval	= 2 min	Hyd. volume	= 7,581 cuft
Drainage area	= 0.641 ac	Curve number	= 90
Basin Slope	= 9.1 %	Hydraulic length	= 375 ft
Tc method	= LAG	Time of conc. (Tc)	= 3.39 min
Total precip.	= 4.58 in	Distribution	= Type III
Storm duration	= 24 hrs	Shape factor	= 484



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Sunday, 01 / 19 / 2020

## Hyd. No. 3

3 Hubbard Road



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Sunday, 01 / 19 / 2020

## Hyd. No. 3

3 Hubbard Road

Hydrograph type Storm frequency Time interval Inflow hyd. No. Reservoir name	<ul> <li>Reservoir</li> <li>1 yrs</li> <li>2 min</li> <li>1 - Proposed Condition - 3 Hu</li> <li>Cultec Chambers</li> </ul>	Peak discharge Time to peak Hyd. volume Mard <b>Be</b> vation Max. Storage	<ul> <li>= 1.205 cfs</li> <li>= 12.13 hrs</li> <li>= 845 cuft</li> <li>= 211.49 ft</li> <li>= 965 cuft</li> </ul>
Reservoir name	= Cultec Chambers	Max. Storage	= 965 CUT



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Sunday, 01 / 19 / 2020

## Hyd. No. 3

3 Hubbard Road



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

Sunday, 01 / 19 / 2020

## Hyd. No. 3

3 Hubbard Road

Hydrograph type Storm frequency Time interval Inflow hyd. No. Reservoir name	<ul> <li>Reservoir</li> <li>2 yrs</li> <li>2 min</li> <li>1 - Proposed Condition - 3 Hu</li> <li>Cultec Chambers</li> </ul>	Peak discharge Time to peak Hyd. volume Mard <b>Be</b> vation Max. Storage	<ul> <li>= 2.039 cfs</li> <li>= 12.07 hrs</li> <li>= 1,688 cuft</li> <li>= 211.60 ft</li> <li>= 990 cuft</li> </ul>
Reservoir name	= Cultec Chambers	Max. Storage	= 990  cuft



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020

#### Sunday, 01 / 19 / 2020

### Hyd. No. 3

3 Hubbard Road

Hydrograph type Storm frequency Time interval Inflow hyd. No.	<ul> <li>Reservoir</li> <li>5 yrs</li> <li>2 min</li> <li>1 - Proposed Condition - 3 Hu</li> <li>Culton Chamborn</li> </ul>	Peak discharge Time to peak Hyd. volume Mard <b>Re</b> vation	= 2.543 cfs = 12.07 hrs = 3,316 cuft = 211.65 ft = 1.002 cuft
Reservoir name	= Cultec Chambers	Max. Storage	= 1,002 cuft



CULTEC	ounder of Storm	Plastic C water and Since	Chamber Techno Septic Solutions e 1986	ology	1-800-4-CU custservice@cult	LTEC ec.com		
Prepared For:	Project Info	rmation:		Engineer:		Calculation	ns Performed B	y:
Andy ayala	Name	3 Hubbard	Rd	Ricardo Ceballos		Ricardo Ce	ballos	
Workforce Partners, LLC				Professional Engineering Cons	sulting services	Professional	Engineering Cons	ulting services
	Wilton	-	1	245 Sturges Hwy		245 Sturges	s Hwy	
Norwalk	CT			Westport		Westport		
CT 06851			10.000	СТ	06880	СТ		06880
Phone	Date:	January	13, 2020	203-635-0922		203-635-09	22	
Fax				mash allas 100 @ amail a		Fax		
Email				rceballos 100@gmail.c	<u>:om</u>	rceballos10	<u>u@gmail.com</u>	
Input Given Parameters							Chamber Sp	ecifications
Unit of Measure	English				Height		30.5	inches
Select Model	Recharge	· 330XLHD			Width		52.00	inches
		_			Length		8.50	feet
Stone Porosity	40.0%			A A A A A A A A A A A A A A A A A A A	Installed Lei	ngth	7.00	feet
Number of Header Systems	1 Header				Bare Chamber	Volume	52.21	cu. feet
Stone Depth <b>Above</b> Chamber	6	inches		and the second second	Installed Chambe	r Volume	79.26	cu. feet
Stone Depth Below Chamber	6	inches		1 Bar	Image for visual ref	erence only.Mag	v not reflect selected	l model.
Workable Bed Depth	6.00	feet			Bed Dept	h	4.63	feet
Max. Bed Width	15.00	feet			Bed Widt	h	11.17	feet
Storage Volume Required	1255.00	cu. feet		$\rightarrow$	Storage Volume	Provided	1282.81	cu. feet
Materials List								
Recharger 330XLHD Stormwater System b	y CULTEC, Inc.							
Approx. Unit Count - not for constructio	n 15	pieces		HVL	V FC-24 Feed Connector	1	pieces	
Actual Number of Chambers Require	d 14	pieces		CULTE	C No. 410™ Filter Fabric	198.42	sq. yards	
Starter Chamber	s <b>2</b>	pieces		CULTEC No	o. 20L Polyethylene Liner	11.17	teet	
Intermediate Chamber	s 10	pieces			Stone	48.98	cu. yards	
End Chamber	s <b>2</b>	pieces						

Bed Detail



Number of Rows Wide	2	pieces
Number of Chambers Long	7	pieces
Chamber Row Width	9.17	feet
Chamber Row Length	50.50	feet
Bed Width	11.17	feet
Bed Length	52.50	feet
Bed Area Required	586.25	sq. feet

Bed detail for reference only. Not project specific. Not to scale. Use CULTEC StormGenie to output project specific detail.

Founder of Plastic Chamber Technology Stormwater and Septic Solutions Since 1986

Project Name: Name

Date:

January 13, 2020

#### Cross Section Detail



Conceptual graphic only. Not job specific.

### Recharger 330XLHD

Pavement	3	inches
95% Compacted Fill	10	inches
Stone Above	6	inches
Chamber Height	30.5	inches
Stone Below	6	inches
Effective Depth	42.5	inches
Bed Depth	55.5	inches





Α	Depth of Stone Base	6.0	inches	Breakdown	of Storage	Provided by
В	Chamber Height	30.5	inches	Recharger 330XLHD	Stormw	ater System
С	Depth of Stone Above Units	6.0	inches	Chambers	753.36	cu. feet
D	Depth of 95% Compacted Fill	10.0	inches	Feed Connectors	0.46	cu. feet
Е	Max. Depth of Cover Allowed Above Crown of Chamber	12.0	feet	Stone	529.00	cu. feet
F	Chamber Width	52.0	inches	Total Storage Provided	1282.81	cu. feet
G	Center to Center Spacing	4.83	feet			

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The Recharger<sup>®</sup> 330XLHD is a 30.5" (775 mm) tall, high capacity chamber. Typically when using this model, fewer chambers are required resulting in less labor and a smaller installation area. The Recharger<sup>®</sup> 330XLHD has the side portal internal manifold feature. HVLV<sup>®</sup> FC-24 Feed Connectors are inserted into the side portals to create the internal manifold.

Size (L x W x H)	8.5' x 52" x 30.5"
	2.59 m x 1321 mm x 775 mm
Installed Length	7'
	2.13 m
Length Adjustment per Run	1.50'
	0.46 m
Chamber Storage	7.46 ft³/ft
	0.69 m³/m
	52.21 ft³/unit
	1.48 m³/unit
Min. Installed Storage	11.32 ft³/ft
	1.05 m³/m
	79.26 ft³/unit
	2.24 m³/unit
Min. Area Required	33.83 ft <sup>2</sup>
	3.14 m <sup>2</sup>
Chamber Weight	73.0 lbs
	33.11 kg
Shipping	30 chambers/skid
	2,335 lbs/skid
	10 skids/48' flatbed
Min. Center-to-Center Spacing	4.83'
	1.47 m
Max. Allowable Cover	12'
	3.66 m
Max. Inlet Opening in End Wall	24" HDPE, PVC
	600 mm HDPE, PVC
Max. Allowable O.D.	10" HDPE, 12" PVC
In Side Portal	250 mm HDPE, 300 mm PVC
Compatible Feed Connector	HVLV FC-24 Feed Connector

Calculations are based on installed chamber length. All above values are nominal.

Min. installed storage includes 6" (152 mm) stone base, 6" (152 mm) stone above crown of chamber and typical stone surround at 58" (1473 mm) center-to-center spacing.

	Stone Foundation Depth					
	6"	12"	18"			
	152 mm	305 mm	457 mm			
Chamber and Stone Storage Per	79.26 ft <sup>3</sup>	86.03 ft <sup>3</sup>	92.79 ft <sup>3</sup>			
	2.24 m <sup>3</sup>	2.44 m <sup>3</sup>	2.63 m <sup>3</sup>			
Min. Effective Depth	3.54'	4.04'	4.54'			
	1.08 m	1.23 m	1.38 m			
Stone Required Per Chamber	2.50 yd <sup>3</sup>	3.13 yd <sup>3</sup>	3.76 yd <sup>3</sup>			
	1.91 m <sup>3</sup>	2.39 m <sup>3</sup>	2.87 m <sup>3</sup>			

#### **Recharger® 330XLHD Bare Chamber Storage Volumes**

Eleva	Elevation		rement Volu	Cumu Stor	lative age		
in.	mm	ft³/ft	m³/m	ft³	m³	ft³	m³
30.5	775	0.000	0.000	0.000	0.000	52.213	1.479
30	762	0.019	0.002	0.133	0.004	52.213	1.479
29	737	0.051	0.005	0.357	0.010	52.080	1.475
28	711	0.084	0.008	0.588	0.017	51.723	1.465
27	686	0.124	0.012	0.868	0.025	51.135	1.448
26	660	0.150	0.014	1.05	0.030	50.267	1.424
25	635	0.173	0.016	1.211	0.034	49.217	1.394
24	609	0.191	0.018	1.337	0.038	48.006	1.360
23	584	0.207	0.019	1.449	0.041	46.669	1.322
22	559	0.221	0.021	1.547	0.044	45.220	1.281
21	533	0.233	0.022	1.631	0.046	43.673	1.237
20	508	0.244	0.023	1.708	0.048	42.042	1.191
19	483	0.254	0.024	1.778	0.050	40.334	1.142
18	457	0.264	0.025	1.848	0.052	38.556	1.092
17	432	0.271	0.025	1.897	0.054	36.708	1.040
16	406	0.283	0.026	1.981	0.056	34.811	0.986
15	381	0.294	0.027	2.058	0.058	32.830	0.930
14	356	0.296	0.027	2.072	0.059	30.772	0.871
13	330	0.299	0.028	2.093	0.059	28.700	0.813
12	305	0.301	0.028	2.107	0.060	26.607	0.754
11	279	0.303	0.028	2.121	0.060	24.500	0.694
10	254	0.304	0.028	2.128	0.060	22.379	0.634
9	229	0.306	0.028	2.142	0.061	20.251	0.574
8	203	0.313	0.029	2.191	0.062	18.109	0.513
7	178	0.321	0.030	2.247	0.064	15.918	0.451
6	152	0.322	0.030	2.254	0.064	13.671	0.387
5	127	0.323	0.030	2.261	0.064	11.417	0.323
4	102	0.324	0.030	2.268	0.064	9.156	0.259
3	76	0.325	0.030	2.275	0.064	6.888	0.195
2	51	0.327	0.030	2.289	0.065	4.613	0.131
1	25	0.332	0.031	2.324	0.066	2.324	0.066
То	tal	7.459	0.693	52.213	1.479	52.213	1.479

Calculations are based on installed chamber length.

### Visit http://cultec.com/downloads/ for Product Downloads and CAD details.

Calculations are based on installed chamber length.

Includes 6" (305 mm) stone above crown of chamber and typical stone surround at 58"(1473 mm) center-to-center spacing and stone foundation as listed in table. Stone void calculated at 40%.



### **Three View Drawing**







CULTEC RECHARGER 330XLHD CHAMBER STORAGE = 7.459 CF/FT [0.693 m³/m] INSTALLED LENGTH ADJUSTMENT = 1.5' [0.46 m] SIDE PORTAL ACCEPTS CULTEC HVLV FC-24 FEED CONNECTOR

## **Typical Interlock Installation**





### **Plan View Drawing**



### **Typical Cross Section for Traffic Application**





## **CULTEC Recharger® 330XLHD Specifications**

#### GENERAL

CULTEC Recharger<sup>®</sup> 330XLHD chambers are designed for underground stormwater management. The chambers may be used for retention, recharging, detention or controlling the flow of on-site stormwater runoff.

#### CHAMBER PARAMETERS

- 1. The chambers shall be manufactured in the U.S.A. by CULTEC, Inc. of Brookfield, CT (cultec.com, 203-775-4416).
- 2. The chamber shall be vacuum thermoformed of polyethylene with a black interior and blue exterior.
- 3. The chamber shall be arched in shape.
- 4. The chamber shall be open-bottomed.
- 5. The chamber shall be joined using an interlocking overlapping rib method. Connections must be fully shouldered overlapping ribs, having no separate couplings or separate end walls.
- 6. The nominal chamber dimensions of the CULTEC Recharger<sup>®</sup> 330XLHD shall be 30.5 inches (775 mm) tall, 52 inches (1321 mm) wide and 8.5 feet (2.59 m) long. The installed length of a joined Recharger<sup>®</sup> 330XLHD shall be 7 feet (2.13 m).
- 7. Maximum inlet opening on the chamber end wall is 24 inches (600 mm) HDPE, PVC.
- 8. The chamber shall have two side portals to accept CULTEC HVLV<sup>®</sup> FC-24 Feed Connectors to create an internal manifold. Maximum allowable O.D. in the side portal is 10 inches (250 mm) HDPE and 12 inches (300 mm) PVC.
- 9. The nominal chamber dimensions of the CULTEC HVLV<sup>®</sup> FC-24 Feed Connector shall be 12 inches (305 mm) tall, 16 inches (406 mm) wide and 24.2 inches (614 mm) long.
- 10. The nominal storage volume of the Recharger<sup>®</sup> 330XLHD chamber shall be 7.459 ft<sup>3</sup> / ft (0.693 m<sup>3</sup> / m) without stone. The nominal storage volume of a single Recharger<sup>®</sup> 330XLRHD Stand Alone unit shall be 63.40 ft<sup>3</sup> (1.80 m<sup>3</sup>) - without stone. The nominal storage volume of a joined Recharger<sup>®</sup> 330XLIHD Intermediate unit shall be 52.213 ft<sup>3</sup> (1.478 m<sup>3</sup>) - without stone. The nominal storage volume of the length adjustment amount per run shall be 11.19 ft<sup>3</sup> (1.04 m<sup>3</sup>) - without stone.
- 11. The nominal storage volume of the HVLV<sup>®</sup> FC-24 Feed Connector shall be 0.913 ft<sup>3</sup> / ft (0.026 m<sup>3</sup> / m) without stone.
- 12. The Recharger<sup>®</sup> 330XLHD chamber shall have fifty-six discharge holes bored into the sidewalls of the unit's core to promote lateral conveyance of water.
- 13. The Recharger<sup>®</sup> 330XLHD chamber shall have 16 corrugations.
- 14. The end wall of the chamber, when present, shall be an integral part of the continuously formed unit. Separate end plates cannot be used with this unit.
- 15. The Recharger<sup>®</sup> 330XLRHD Stand Alone unit must be formed as a whole chamber having two fully formed integral end walls and having no separate end plates or separate end walls.
- 16. The Recharger<sup>®</sup> 330XLSHD Starter unit must be formed as a whole chamber having one fully formed integral end wall and one partially formed integral end wall with a lower transfer opening of 14 inches (356 mm) high x 34.5 inches (876 mm) wide.
- 17. The Recharger<sup>®</sup> 330XLIHD Intermediate unit must be formed as a whole chamber having one fully open end wall and one partially formed integral end wall with a lower transfer opening of 14 inches (356 mm) high x 34.5 inches (876 mm) wide.
- 18. The Recharger<sup>®</sup> 330XLEHD End unit must be formed as a whole chamber having one fully formed integral end wall and one fully open end wall and having no separate end plates or end walls.
- 19. The HVLV® FC-24 Feed Connector must be formed as a whole chamber having two open end walls and having no separate end plates or separate end walls. The unit shall fit into the side portals of the Recharger® 330XLHD and act as cross feed connections.
- 20. Chambers must have horizontal stiffening flex reduction steps between the ribs.
- 21. The chamber shall have a raised integral cap at the top of the arch in the center of each unit to be used as an optional inspection port or clean-out.
- 22. The units may be trimmed to custom lengths by cutting back to any corrugation on the large rib end.
- 23. The chamber shall be manufactured in an ISO 9001:2015 certified facility.
- 24. Maximum allowable cover over the top of the chamber shall be 12' (3.66 m).
- 25. The chamber shall be designed to withstand traffic loads when installed according to CULTEC's recommended installation instructions.

APPENDIX G



NOAA Atlas 14, Volume 10, Version 3 Location name: Wilton, Connecticut, USA\* Latitude: 41.1936°, Longitude: -73.4339° Elevation: 210.34 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

### PF\_tabular | PF\_graphical | Maps\_&\_aerials

### PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches) <sup>1</sup>										
Duration				Average	recurrence	interval (ye	ars)			
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	<b>0.364</b> (0.282-0.462)	<b>0.424</b> (0.328-0.539)	<b>0.522</b> (0.403-0.667)	<b>0.604</b> (0.463-0.774)	<b>0.716</b> (0.532-0.951)	<b>0.801</b> (0.582-1.08)	<b>0.889</b> (0.627-1.24)	<b>0.983</b> (0.661-1.40)	<b>1.11</b> (0.720-1.63)	<b>1.21</b> (0.768-1.82)
10-min	<b>0.515</b> (0.399-0.655)	<b>0.600</b> (0.465-0.764)	<b>0.739</b> (0.569-0.943)	<b>0.855</b> (0.656-1.10)	<b>1.01</b> (0.753-1.35)	<b>1.14</b> (0.826-1.53)	<b>1.26</b> (0.888-1.75)	<b>1.39</b> (0.938-1.99)	<b>1.58</b> (1.02-2.31)	<b>1.72</b> (1.09-2.57)
15-min	<b>0.606</b> (0.469-0.771)	<b>0.706</b> (0.547-0.899)	<b>0.870</b> (0.671-1.11)	<b>1.01</b> (0.772-1.29)	<b>1.19</b> (0.886-1.59)	<b>1.34</b> (0.970-1.80)	<b>1.48</b> (1.05-2.06)	<b>1.64</b> (1.10-2.33)	<b>1.85</b> (1.20-2.72)	<b>2.02</b> (1.28-3.03)
30-min	<b>0.848</b> (0.657-1.08)	<b>0.986</b> (0.763-1.25)	<b>1.21</b> (0.935-1.55)	<b>1.40</b> (1.07-1.80)	<b>1.66</b> (1.23-2.20)	<b>1.86</b> (1.35-2.50)	<b>2.06</b> (1.44-2.85)	<b>2.26</b> (1.52-3.22)	<b>2.54</b> (1.65-3.73)	<b>2.75</b> (1.74-4.12)
60-min	<b>1.09</b> (0.844-1.39)	<b>1.26</b> (0.979-1.61)	<b>1.55</b> (1.20-1.98)	<b>1.79</b> (1.38-2.30)	<b>2.12</b> (1.57-2.81)	<b>2.38</b> (1.72-3.20)	<b>2.63</b> (1.84-3.63)	<b>2.89</b> (1.94-4.11)	<b>3.22</b> (2.09-4.73)	<b>3.48</b> (2.20-5.20)
2-hr	<b>1.40</b> (1.09-1.77)	<b>1.65</b> (1.28-2.08)	<b>2.05</b> (1.59-2.60)	<b>2.39</b> (1.84-3.04)	<b>2.85</b> (2.13-3.77)	<b>3.20</b> (2.34-4.31)	<b>3.57</b> (2.53-4.95)	<b>3.97</b> (2.68-5.62)	<b>4.52</b> (2.94-6.60)	<b>4.97</b> (3.16-7.39)
3-hr	<b>1.61</b> (1.26-2.02)	<b>1.90</b> (1.49-2.40)	<b>2.39</b> (1.86-3.02)	<b>2.80</b> (2.16-3.55)	<b>3.35</b> (2.51-4.42)	<b>3.77</b> (2.77-5.06)	<b>4.21</b> (3.01-5.84)	<b>4.71</b> (3.19-6.65)	<b>5.42</b> (3.53-7.89)	<b>6.01</b> (3.82-8.90)
6-hr	<b>2.02</b> (1.59-2.53)	<b>2.41</b> (1.90-3.02)	<b>3.06</b> (2.40-3.84)	<b>3.59</b> (2.80-4.53)	<b>4.33</b> (3.27-5.68)	<b>4.88</b> (3.61-6.52)	<b>5.46</b> (3.93-7.55)	<b>6.14</b> (4.17-8.61)	<b>7.13</b> (4.66-10.3)	<b>7.96</b> (5.08-11.7)
12-hr	<b>2.50</b> (1.98-3.10)	<b>3.00</b> (2.37-3.73)	<b>3.81</b> (3.01-4.75)	<b>4.49</b> (3.52-5.63)	<b>5.43</b> (4.12-7.08)	<b>6.13</b> (4.56-8.14)	<b>6.87</b> (4.97-9.45)	<b>7.73</b> (5.27-10.8)	<b>9.00</b> (5.90-12.9)	<b>10.1</b> (6.44-14.7)
24-hr	<b>2.93</b> (2.33-3.61)	<b>3.55</b> (2.83-4.39)	<b>4.58</b> (3.63-5.66)	<b>5.42</b> (4.27-6.75)	<b>6.59</b> (5.03-8.55)	<b>7.46</b> (5.58-9.87)	<b>8.39</b> (6.11-11.5)	<b>9.49</b> (6.49-13.1)	<b>11.1</b> (7.32-15.9)	<b>12.5</b> (8.04-18.2)
2-day	<b>3.27</b> (2.62-4.00)	<b>4.03</b> (3.23-4.94)	<b>5.29</b> (4.22-6.50)	<b>6.33</b> (5.02-7.82)	<b>7.76</b> (5.97-10.0)	<b>8.82</b> (6.65-11.6)	<b>9.97</b> (7.33-13.7)	<b>11.4</b> (7.80-15.6)	<b>13.5</b> (8.91-19.2)	<b>15.4</b> (9.89-22.1)
3-day	<b>3.53</b> (2.84-4.30)	<b>4.37</b> (3.51-5.34)	<b>5.75</b> (4.60-7.04)	<b>6.89</b> (5.48-8.48)	<b>8.47</b> (6.53-10.9)	<b>9.63</b> (7.29-12.7)	<b>10.9</b> (8.03-14.9)	<b>12.4</b> (8.55-17.1)	<b>14.8</b> (9.79-20.9)	<b>16.9</b> (10.9-24.2)
4-day	<b>3.78</b> (3.05-4.60)	<b>4.67</b> (3.76-5.69)	<b>6.13</b> (4.92-7.48)	<b>7.34</b> (5.85-9.00)	<b>9.00</b> (6.96-11.5)	<b>10.2</b> (7.75-13.4)	<b>11.6</b> (8.54-15.7)	<b>13.2</b> (9.09-18.0)	<b>15.7</b> (10.4-22.1)	<b>17.8</b> (11.5-25.5)
7-day	<b>4.51</b> (3.65-5.46)	<b>5.48</b> (4.44-6.64)	<b>7.08</b> (5.71-8.59)	<b>8.40</b> (6.73-10.2)	<b>10.2</b> (7.92-13.0)	<b>11.6</b> (8.78-15.0)	<b>13.0</b> (9.61-17.5)	<b>14.7</b> (10.2-20.0)	<b>17.3</b> (11.5-24.2)	<b>19.5</b> (12.6-27.8)
10-day	<b>5.23</b> (4.25-6.30)	<b>6.25</b> (5.08-7.55)	<b>7.93</b> (6.42-9.60)	<b>9.33</b> (7.50-11.3)	<b>11.2</b> (8.74-14.2)	<b>12.7</b> (9.64-16.3)	<b>14.2</b> (10.5-18.9)	<b>16.0</b> (11.1-21.6)	<b>18.5</b> (12.3-25.8)	<b>20.7</b> (13.4-29.3)
20-day	<b>7.39</b> (6.05-8.85)	<b>8.55</b> (6.98-10.2)	<b>10.4</b> (8.49-12.5)	<b>12.0</b> (9.71-14.5)	<b>14.2</b> (11.0-17.7)	<b>15.8</b> (12.0-20.1)	<b>17.5</b> (12.8-22.9)	<b>19.3</b> (13.4-25.9)	<b>21.8</b> (14.6-30.1)	<b>23.8</b> (15.5-33.5)
30-day	<b>9.18</b> (7.54-11.0)	<b>10.4</b> (8.55-12.4)	<b>12.5</b> (10.2-14.9)	<b>14.2</b> (11.5-17.0)	<b>16.5</b> (12.9-20.5)	<b>18.3</b> (13.9-23.0)	<b>20.1</b> (14.7-26.0)	<b>21.9</b> (15.3-29.2)	<b>24.3</b> (16.3-33.5)	<b>26.2</b> (17.1-36.8)
45-day	<b>11.4</b> (9.39-13.5)	<b>12.7</b> (10.5-15.1)	<b>14.9</b> (12.3-17.8)	<b>16.8</b> (13.7-20.1)	<b>19.3</b> (15.1-23.8)	<b>21.2</b> (16.2-26.6)	<b>23.2</b> (17.0-29.8)	<b>25.1</b> (17.6-33.3)	<b>27.5</b> (18.5-37.6)	<b>29.2</b> (19.1-40.8)
60-day	<b>13.2</b> (10.9-15.7)	<b>14.7</b> (12.1-17.4)	<b>17.0</b> (14.0-20.2)	<b>18.9</b> (15.5-22.6)	<b>21.6</b> (17.0-26.6)	<b>23.7</b> (18.1-29.6)	<b>25.7</b> (18.9-32.9)	<b>27.6</b> (19.4-36.6)	<b>30.1</b> (20.2-41.1)	<b>31.8</b> (20.8-44.3)

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

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**PF** graphical





Duration 5-min 2-day 10-min 3-day 15-min 4-day 30-min 7-day 60-min 10-day 20-day 2-hr 30-day 3-hr 6-hr 45-day 12-hr 60-day 24-hr

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Maps & aerials

Small scale terrain



Large scale terrain



Large scale map Worcest hamton Springfield Prov Hartford Rhoo Connecticut Island Waterbury 84 Scranton 87 Bridgeport Long Island Sound New Jersey 80 6, New York New York +Allentown 78 Edison \_ ing 100km 60mi River

Large scale aerial



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NOAA Atlas 14, Volume 10, Version 3 Location name: Wilton, Connecticut, USA\* Latitude: 41.1936°, Longitude: -73.4339° Elevation: 210.34 ft\*\* \* source: ESRI Maps \*\* source: USGS



#### POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

### PF\_tabular | PF\_graphical | Maps\_&\_aerials

### PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour) <sup>1</sup>												
Duration		Average recurrence interval (years)										
Duration	1	2	5	10	25	50	100	200	500	1000		
5-min	<b>4.37</b>	<b>5.09</b>	<b>6.26</b>	<b>7.25</b>	<b>8.59</b>	<b>9.61</b>	<b>10.7</b>	<b>11.8</b>	<b>13.3</b>	<b>14.6</b>		
	(3.38-5.54)	(3.94-6.47)	(4.84-8.00)	(5.56-9.29)	(6.38-11.4)	(6.98-13.0)	(7.52-14.8)	(7.93-16.8)	(8.64-19.6)	(9.22-21.8)		
10-min	<b>3.09</b>	<b>3.60</b>	<b>4.43</b>	<b>5.13</b>	<b>6.08</b>	<b>6.82</b>	<b>7.56</b>	<b>8.36</b>	<b>9.46</b>	<b>10.3</b>		
	(2.39-3.93)	(2.79-4.58)	(3.41-5.66)	(3.94-6.58)	(4.52-8.08)	(4.96-9.20)	(5.33-10.5)	(5.63-11.9)	(6.13-13.9)	(6.53-15.4)		
15-min	<b>2.42</b>	<b>2.82</b>	<b>3.48</b>	<b>4.02</b>	<b>4.77</b>	<b>5.34</b>	<b>5.93</b>	<b>6.55</b>	<b>7.41</b>	<b>8.09</b>		
	(1.88-3.08)	(2.19-3.60)	(2.68-4.44)	(3.09-5.16)	(3.54-6.34)	(3.88-7.22)	(4.18-8.24)	(4.41-9.34)	(4.81-10.9)	(5.12-12.1)		
30-min	<b>1.70</b>	<b>1.97</b>	<b>2.42</b>	<b>2.80</b>	<b>3.32</b>	<b>3.71</b>	<b>4.11</b>	<b>4.52</b>	<b>5.08</b>	<b>5.50</b>		
	(1.31-2.16)	(1.53-2.51)	(1.87-3.09)	(2.15-3.59)	(2.46-4.39)	(2.69-5.00)	(2.89-5.69)	(3.04-6.44)	(3.29-7.45)	(3.48-8.23)		
60-min	<b>1.09</b>	<b>1.26</b>	<b>1.55</b>	<b>1.79</b>	<b>2.12</b>	<b>2.38</b>	<b>2.63</b>	<b>2.89</b>	<b>3.22</b>	<b>3.48</b>		
	(0.844-1.39)	(0.979-1.61)	(1.20-1.98)	(1.38-2.30)	(1.57-2.81)	(1.72-3.20)	(1.84-3.63)	(1.94-4.11)	(2.09-4.73)	(2.20-5.20)		
2-hr	<b>0.698</b>	<b>0.822</b>	<b>1.03</b>	<b>1.19</b>	<b>1.43</b>	<b>1.60</b>	<b>1.78</b>	<b>1.98</b>	<b>2.26</b>	<b>2.49</b>		
	(0.544-0.882)	(0.640-1.04)	(0.796-1.30)	(0.921-1.52)	(1.07-1.88)	(1.17-2.15)	(1.27-2.47)	(1.34-2.81)	(1.47-3.30)	(1.58-3.70)		
3-hr	<b>0.535</b>	<b>0.634</b>	<b>0.796</b>	<b>0.931</b>	<b>1.12</b>	<b>1.26</b>	<b>1.40</b>	<b>1.57</b>	<b>1.81</b>	<b>2.00</b>		
	(0.418-0.673)	(0.495-0.799)	(0.620-1.01)	(0.721-1.18)	(0.837-1.47)	(0.923-1.69)	(1.00-1.95)	(1.06-2.21)	(1.18-2.63)	(1.27-2.96)		
6-hr	<b>0.337</b>	<b>0.403</b>	<b>0.511</b>	<b>0.600</b>	<b>0.723</b>	<b>0.814</b>	<b>0.912</b>	<b>1.02</b>	<b>1.19</b>	<b>1.33</b>		
	(0.265-0.422)	(0.317-0.505)	(0.400-0.641)	(0.467-0.756)	(0.545-0.948)	(0.602-1.09)	(0.656-1.26)	(0.696-1.44)	(0.778-1.72)	(0.848-1.95)		
12-hr	<b>0.207</b>	<b>0.249</b>	<b>0.317</b>	<b>0.373</b>	<b>0.451</b>	<b>0.508</b>	<b>0.570</b>	<b>0.642</b>	<b>0.747</b>	<b>0.836</b>		
	(0.164-0.257)	(0.197-0.309)	(0.249-0.395)	(0.292-0.467)	(0.342-0.587)	(0.378-0.676)	(0.412-0.784)	(0.437-0.894)	(0.490-1.07)	(0.535-1.22)		
24-hr	<b>0.122</b>	<b>0.148</b>	<b>0.191</b>	<b>0.226</b>	<b>0.275</b>	<b>0.311</b>	<b>0.350</b>	<b>0.396</b>	<b>0.464</b>	<b>0.522</b>		
	(0.097-0.150)	(0.118-0.183)	(0.151-0.236)	(0.178-0.281)	(0.210-0.356)	(0.233-0.411)	(0.255-0.479)	(0.270-0.548)	(0.305-0.662)	(0.335-0.758)		
2-day	<b>0.068</b>	<b>0.084</b>	<b>0.110</b>	<b>0.132</b>	<b>0.162</b>	<b>0.184</b>	<b>0.208</b>	<b>0.237</b>	<b>0.282</b>	<b>0.320</b>		
	(0.054-0.083)	(0.067-0.103)	(0.088-0.135)	(0.104-0.163)	(0.124-0.209)	(0.139-0.242)	(0.153-0.284)	(0.162-0.326)	(0.186-0.399)	(0.206-0.461)		
3-day	<b>0.049</b>	<b>0.061</b>	<b>0.080</b>	<b>0.096</b>	<b>0.118</b>	<b>0.134</b>	<b>0.151</b>	<b>0.173</b>	<b>0.206</b>	<b>0.234</b>		
	(0.039-0.060)	(0.049-0.074)	(0.064-0.098)	(0.076-0.118)	(0.091-0.151)	(0.101-0.176)	(0.112-0.207)	(0.119-0.237)	(0.136-0.291)	(0.151-0.336)		
4-day	<b>0.039</b>	<b>0.049</b>	<b>0.064</b>	<b>0.076</b>	<b>0.094</b>	<b>0.107</b>	<b>0.120</b>	<b>0.137</b>	<b>0.163</b>	<b>0.186</b>		
	(0.032-0.048)	(0.039-0.059)	(0.051-0.078)	(0.061-0.094)	(0.072-0.120)	(0.081-0.140)	(0.089-0.164)	(0.095-0.188)	(0.108-0.230)	(0.120-0.266)		
7-day	<b>0.027</b>	<b>0.033</b>	<b>0.042</b>	<b>0.050</b>	<b>0.061</b>	<b>0.069</b>	<b>0.077</b>	<b>0.088</b>	<b>0.103</b>	<b>0.116</b>		
	(0.022-0.032)	(0.026-0.040)	(0.034-0.051)	(0.040-0.061)	(0.047-0.077)	(0.052-0.089)	(0.057-0.104)	(0.061-0.119)	(0.068-0.144)	(0.075-0.165)		
10-day	<b>0.022</b>	<b>0.026</b>	<b>0.033</b>	<b>0.039</b>	<b>0.047</b>	<b>0.053</b>	<b>0.059</b>	<b>0.066</b>	<b>0.077</b>	<b>0.086</b>		
	(0.018-0.026)	(0.021-0.031)	(0.027-0.040)	(0.031-0.047)	(0.036-0.059)	(0.040-0.068)	(0.044-0.079)	(0.046-0.090)	(0.051-0.108)	(0.056-0.122)		
20-day	<b>0.015</b>	<b>0.018</b>	<b>0.022</b>	<b>0.025</b>	<b>0.029</b>	<b>0.033</b>	<b>0.036</b>	<b>0.040</b>	<b>0.045</b>	<b>0.050</b>		
	(0.013-0.018)	(0.015-0.021)	(0.018-0.026)	(0.020-0.030)	(0.023-0.037)	(0.025-0.042)	(0.027-0.048)	(0.028-0.054)	(0.030-0.063)	(0.032-0.070)		
30-day	<b>0.013</b>	<b>0.014</b>	<b>0.017</b>	<b>0.020</b>	<b>0.023</b>	<b>0.025</b>	<b>0.028</b>	<b>0.030</b>	<b>0.034</b>	<b>0.036</b>		
	(0.010-0.015)	(0.012-0.017)	(0.014-0.021)	(0.016-0.024)	(0.018-0.028)	(0.019-0.032)	(0.020-0.036)	(0.021-0.041)	(0.023-0.047)	(0.024-0.051)		
45-day	<b>0.011</b>	<b>0.012</b>	<b>0.014</b>	<b>0.016</b>	<b>0.018</b>	<b>0.020</b>	<b>0.021</b>	<b>0.023</b>	<b>0.025</b>	<b>0.027</b>		
	(0.009-0.013)	(0.010-0.014)	(0.011-0.016)	(0.013-0.019)	(0.014-0.022)	(0.015-0.025)	(0.016-0.028)	(0.016-0.031)	(0.017-0.035)	(0.018-0.038)		
60-day	<b>0.009</b> (0.008-0.011)	<b>0.010</b> (0.008-0.012)	<b>0.012</b> (0.010-0.014)	<b>0.013</b> (0.011-0.016)	<b>0.015</b> (0.012-0.018)	<b>0.016</b> (0.013-0.021)	<b>0.018</b> (0.013-0.023)	<b>0.019</b> (0.014-0.025)	<b>0.021</b> (0.014-0.029)	<b>0.022</b> (0.014-0.031)		

<sup>1</sup> Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

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### **PF** graphical





Duration								
5-min	2-day							
10-min	— 3-day							
15-min	— 4-day							
30-min	- 7-day							
- 60-min	— 10-day							
— 2-hr	— 20-day							
— 3-hr	— 30-day							
— 6-hr	— 45-day							
- 12-hr	- 60-day							
24-hr								

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Large scale terrain



Large scale map Worcest hamton Springfield Prov Hartford Rhoo Connecticut Island Waterbury 84 Scranton 87 Bridgeport Long Island Sound New Jersey 80 6, New York New York +Allentown 78 Edison \_ ing 100km 60mi River

Large scale aerial



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APPENDIX H

### Maintenance & Inspection Schedule 3 Hubbard Rd, Wilton CT

Regular inspection and maintenance of the stormwater management system and uphill areas is necessary to ensure proper operation. Inspections of the stormwater management system and pavement areas should be conducted monthly based on the following table:

## Site Areas:

<b>Inspection</b>	and	Maintena	nce
01			0

Check for:	Corrective Measure:
Erosion	Install erosion control measures and provide stabilization measures
Spillage	Contain spill as close to source as possible with a dike of absorbent materials installed to protect drainage inlets, stormwater areas, or downstream wetlands and streams. All hazardous waste material, including absorbent materials must be disposed of by a licensed hazardous waste transporter and disposed of in an environmentally acceptable manner
Sediment Accumulation	Stabilize any disturbed areas uphill of where the sedimentation is occurring. Use temporary erosion control measures (i.e. silt fence, straw bales) to filter stormwater runoff.
Trash	Pick up and dispose of trash and litter in an environmentally acceptable manner.

At a minimum the following maintenance measures shall be provided at the frequency listed in the following table:

Maintenance Measure:	Frequency:
Pavement Sweeping	Minimum two times per year: during spring cleanup (after last snow event)
	and during fall cleanup (to remove fallen leaves)
Pavement De-icing	Apply anti-icing treatment prior to storms
	Apply deicing treatments as needed during and after snow storms and mixed
	precipitation events to control ice and compact snow not removed during
	plowing

# Catch Basins and Pipe:

Inspection and Maintenance	
Check for:	Corrective Measure:
Trash, Sediment, and	Remove trash, sediment, and debris and dispose of in an environmentally
Debris at Grate	acceptable manner.
Sediment & Trash	Remove sediment from sumps if depth of deposits is greater than one-half
Accumulation in Sump	the depth from the bottom of the catch basin to the invert of the lowest pipe
	in the basin.
Pipe blockages	Flush pipes to remove blockages. TV inspect as required.

At a minimum the following maintenance measures shall be provided at the frequency listed in the following table:

### Routine Maintenance

Maintenance Measure:	Frequency:
Sediment Removal	Minimum one time per year: Remove sediment and trash from catch basin
	sumps and grates and pipe inverts. Dispose of sediment and trash in an
	environmentally acceptable manner.

### Maintenance & Inspection Schedule

### Infiltration System Inspection and Maintenance

Check for:	Corrective Measure:
Sediment Accumulation	Remove sediment when accumulation exceeds 3 inches throughout the
	length of the row per manufacturer's specifications.
Trash and Debris	Remove trash and debris and dispose of in an environmentally acceptable
	manner.

At a minimum the following maintenance measures shall be provided at the frequency listed in the following table:

### **Routine Maintenance**

Maintenance Measure:	Frequency:
Sediment Removal	Inspected a minimum immediately after completion of the site's construction,
	every 6 months for the first year of operation and annually after the end of the
	first year. Remove sediment accumulation as required. Dispose of sediment
	in an environmentally acceptable manner.
Trash and Debris	Minimum once per year.

The infiltration system is to be inspected and maintained per manufacturer's specifications. Product information is attached.
			Inspector:
Date:	Time:		Site Conditions:
Date Since Last Precipitation Event:			
Inspection Item	Satisfa	actory?	<b>Comments or Corrective Measures</b>
	Yes (Y) o	or No (N)	Taken
Site Areas			
Erosion	~	z	
Spillage	~	Z	
Sediment Accumulation	~	Z	
Trash	~	Z	
Catch Basins and Pipe			
Trash, Sediment, and Debris at Inlet Grates	~	Z	
Sediment & Trash Accumulation in Sump	≻	Z	
Pipe blockages	≻	Z	
Infiltration System			
Sediment Accumulation	≻	z	
Trash and Debris	>	Z	

## STORMWATER MANAGEMENT SYSTEM INSPECTION AND MAINTENANCE CHECKLIST