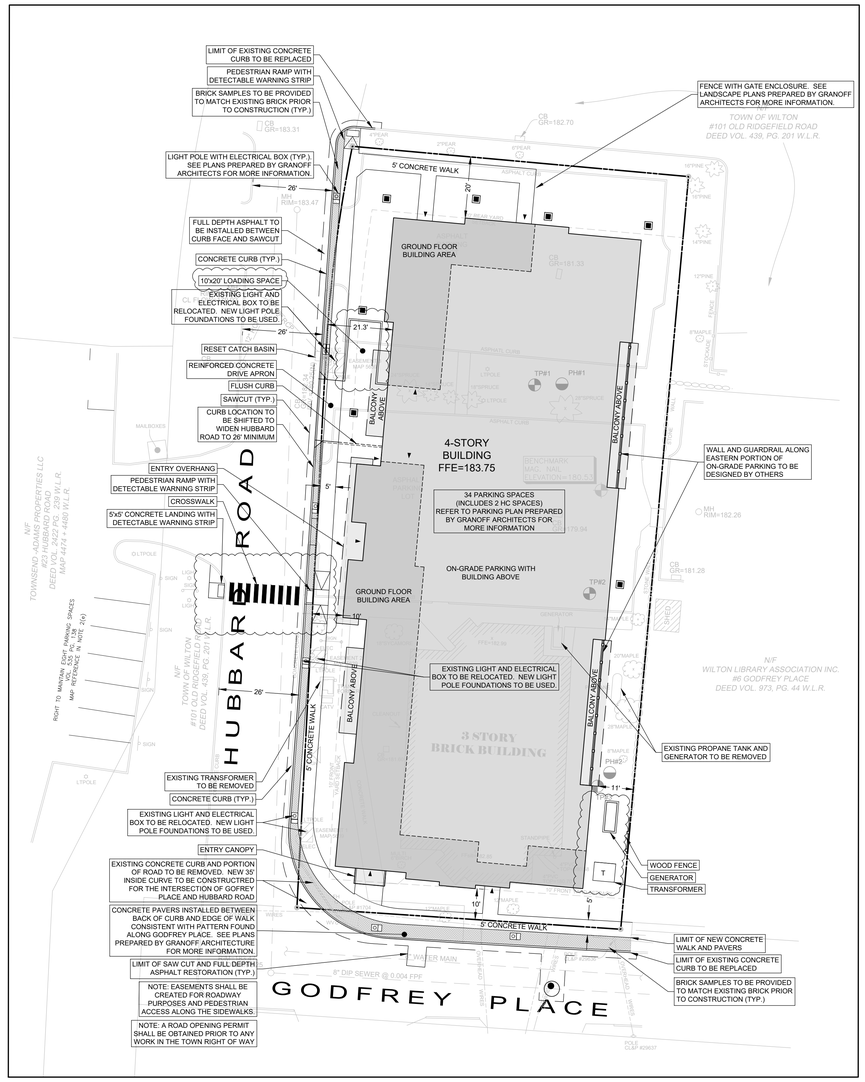
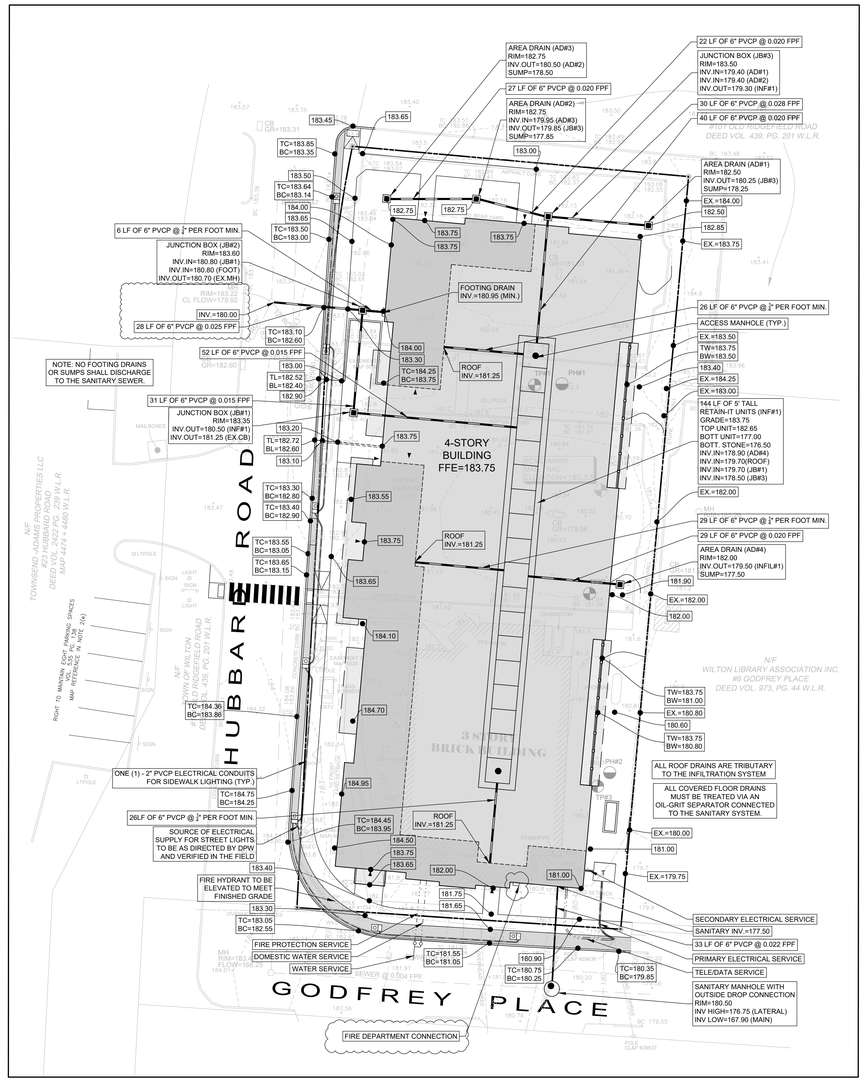


SITE PLAN INSET



GRADING & DRAINAGE INSET

AREA & BULK CALCULATIONS

	Standard	Standards Per Wilton Zoning WC 29-6.E	Proposed Standards Per CGS Sec. 8-30g
1	Minimum Front Yard	10'	10.0'
2	Maximum Front Yard	20'	21.3"
3	Minimum Side Yard (Each)	0'	11.0'
4	Minimum Rear Yard	20'	20.0'
5	Minimum Parking & Loading Setbacks (side & rear yards)	0'	14.4' / 36'
6	Maximum Building Height (Stories/Feet)	3 / 42'	5 Stories / 62.5'[1]
7	Maximum Building coverage (%)	30	64
8	Maximum Site Coverage (%)	80	75
9	Minimum Lot Size (acres)	No Minimum	0.625 acres (27,246 sf)
12	Maximum Floor Area Ratio (F.A.R)	N/A	2.50[2]
	Maximum Density - (29-6.C.4.b) (Multi-Family)	5 Units / Ac	42 Units (67 Units / Ac)
	Required Affordable Housing Unit	None	30% or 13 Units (Meeting 8-30g Reqs.)

NOTES:
1. Calculated average grade of 183.10
2. Information from Granoff Architects

Parking Calculations

Use	Rate Per Sec 29-8.B Wilton Zoning	Quantity	Total
Studio or 1-Bedroom Unit (29-8.b.5.a(2))	1.0 / Unit	13 Units	13 Spaces
2 & 3-Bedroom Unit (29-8.b.5.a(2))	2.0 / Unit	29 Units	58 Spaces
Parking Standard Per Wilton Zoning Regs. 29-8.B			**71 Spaces**
Parking Provided (1 / Unit) Per CGS 8-30g			**42 Spaces[1]**

NOTES:
1. Includes 8 spaces maintained on 23 Hubbard Road property as depicted in deed of record Vol. 522, Pg. 143

GENERAL NOTES:

1. These drawings are intended only to depict the design of site grading, drainage, sanitary, utilities and sediment & erosion controls. These drawings are for approval purposes only. No construction may begin prior to obtaining all necessary permits and approvals.
2. All survey data, boundary lines, topography, building locations and area calculations are from a survey prepared by Redniss & Mead, Inc. entitled Property & Topographic Survey dated April 22, 2022 and revised June 9, 2022. Elevations depicted or labeled are based on NAVD-88.
3. Refer to plans prepared by Granoff Architects for information and design of the proposed buildings. These drawings depict site plans corresponding to the latest architectural plans received from Granoff Architects received on August 30, 2022.
4. Property lies in the Wilton Center District Zone.
5. All construction shall comply with the Town of Wilton requirements, the State of Connecticut Basic Building Code Americans with Disabilities Act (ADA), the Connecticut Guidelines for Soil and Erosion and Sediment Control, OSHA, and CT DOT Form 818 (latest edition).
6. All development activities to be undertaken within the street right-of-way and other public lands shall comply fully with Town standards unless approved deviation is specifically set forth as part of this application. All work within the State right-of-way will comply with the CT DOT Form 818 with the latest special Provisions and Typical State Standard Details.
7. Contractor shall supply complete shop drawings including manufacturer's product data sheets to the Site Engineer, for all construction material used in conjunction with these drawings. Contractor shall allow a 5 day review period, prior to fabrication and installation.
8. Information on existing utilities has been compiled from various sources including utility company records, municipal record maps and field survey and is not guaranteed to be correct or complete. The contractor is solely responsible for determining actual locations and elevations of all utilities including underground services.
9. The property is served by public water and sewer system.
10. Prior to any excavation the Contractor and/or Applicant, in accordance with Public Act 77-350, shall be required to contact "Call Before You Dig" at 1-800-922-4455 for mark-out of underground utilities. Dig test pit(s) at utility crossing(s) to check actual clearances with new utilities prior to construction. If conflicts are found the contractor shall notify the engineer, at which time the sewer in question shall be redesigned. If such redesign is not possible, the existing pipes or utilities shall be relocated to avoid the conflict. Such relocation shall be done with knowledge of and in accordance with the owner of the utility.
11. It shall be the responsibility of the contractor to provide any excavation safeguards, necessary barricades, flagmen, etc., for traffic control and site safety. All work shall be done in accordance with OSHA requirements. The contractor shall be responsible for compliance with OSHA requirements.
12. When preparing the existing site for the proposed development, all materials removed shall be disposed of in conformance with all governing agencies.
13. Remove stumps and brush from site, or chip and use during landscaping. Do not bury stumps on site.
14. Building elevations are subject to change and shall be finalized prior to building permit.
15. Special attention of the contractor is called to the required type and compaction of pipe bedding and backfill specified on these drawings. These requirements will be strictly enforced.
16. Prior to issuance of a Certificate of Occupancy, the Engineering Bureau may require a certification letter stating that the development was constructed in accordance to the approved plans, and an "as-built" drawing shall be submitted
17. The Contractor is responsible for coordinating with a licensed surveyor to prepare an "as-built" plan. The Contractor is responsible to coordinate with a site engineer 48 hours prior to any inspections.
18. The Engineering Department and the inspecting engineer shall be notified by the contractor three (3) days prior to the commencement of each phase of construction.
19. The work shall be done in conformance with the contract documents/plans unless changes have been approved in writing by the design engineer prior to the work being done.
20. A preconstruction meeting shall be held with the Owner, Architect and Engineer to review the scope of construction. The Contractor shall be responsible to coordinate the preconstruction meeting.

EARTHWORK & GRADING:

21. Grade away from building walls at 2% minimum (typical).
22. Earth slopes shall be no steeper than 2:1 (horz.:vert.)
23. General fill beyond paved areas shall be free of brush rubbish, stumps and stones larger than 8". Fill shall be placed in compacted layers not to exceed 8" in thickness. The dry density after compaction shall not be less than 95% of the Standard Proctor Test and done in accordance with the requirements of ASTM D698. After compacting, the fill shall be 4" below the required grade as shown on the plan.
24. General fill may be till, loam, sand or gravel mixture classified as SP, SW, SM, GP, GM, ML per the United Soil Classification System. It shall have not more than 40% fines passing the #100 sieve, not more than 8% passing the #200 sieve, and no stones larger than 8".
25. Subgrade and fill shall be uniformly compacted by the use of equipment manufactured for that purpose. Rollers shall deliver a ground pressure of not less than 300 pounds per linear inch of contact width and weigh not less than 10 tons. Vibratory units shall have a static weight of not less than 4 tons. The amount of compactive effort shall be as directed by the Engineer, but in no case shall be less than 4 complete passes of the compacting equipment being used.
26. Disturbed areas shall be top soiled, seeded with grass and mulched in a manner conforming to the recommendations of the "Guidelines for Soil Erosion and Sediment Control", published by The Connecticut Council on Soil and Water Conservation, May 2002.
27. After the areas to be topsoiled have been brought to grade, the subgrade shall be loosened by scarifying to a depth of at least 2" to ensure bonding of the topsoil and subsoil.
28. Topsoil shall be friable and loamy with high organic content. It shall be free of debris, rocks larger than 2" and roots. Topsoil shall have at least 1.5 percent by weight of fine textured stable organic material and no greater than 6 percent. Topsoil shall not have less than 20% fine textured material (passing the No. 200 sieve) and not more than 15% clay. pH range shall be 6.0-7.5 and soluble salts shall not exceed 500ppm.
29. Fill or topsoil shall not be placed nor compacted while in a frozen or muddy condition or while subgrade is frozen.
30. Excavation for pipes or concrete pavement repair may require either a braced excavation or open cut designed according to the requirements of OSHA, 29 CFR Part 1926. The lateral support systems and slopes should also be designed such that building footings, slabs on grade, adjacent pavement and existing ut ilities are protected and supported and not allowed to settle. The contractor shall be responsible for having a Professional Engineer, registered in the State of Connecticut design the excavation support method. The designs shall be submitted to the owner or his geotechnical engineer for review. The contractor shall submit plans showing the type, limits, design and sequence of construction for the lateral support system.
31. During the excavation, it is anticipated that existing utilities and sewers may be exposed. The contractor shall provide protection and support of these facilities and repair any damage caused by the work in a manner satisfactory to the owner. The condition of the existing facilities shall be observed by the owner's representative who shall determine if the facilities shall be replaced. Replacement of the facilities shall be done in a manner satisfactory to the owner and in compliance with applicable Codes.

STORM AND SANITARY SEWER SYSTEMS:

32. All pipe shall be installed straight and at the vertical and horizontal alignment shown. Pipes shall have a uniform slope as specified.
33. Minimum cover on all pipes shall be two feet (2') unless otherwise noted.
34. All storm pipe specified as Poly Vinyl Chloride Pipe (PVCP) shall be SDR 35 with rubber gasketed joints and meet the requirements of ASTM D3034 and D3312.
35. All High Density Polyethylene Pipe (HDPE) for the stormwater system shall be ADS N-12 or equivalent with O-Ring joints (Pro-series) suitable for water tight installations.
36. All sanitary sewer pipe shall be Poly Vinyl Chloride Pipe (PVCP) and shall be Schedule 40 with solvent weld joints.
37. Dig test pits at utility and sewer crossings to check actual clearances with these facilities prior to construction. Dig test pits at the connection points to existing sanitary sewer pipes to confirm that the elevation of the proposed gravity sewer is appropriate. If conflicts are found the contractor shall notify the engineer at which time the sewer in question shall be redesigned. If such redesign is not possible, the existing pipes or utilities shall be relocated to avoid conflict.
38. All area drains shall have a two foot (2') sump with bell traps or 90° PVC elbows.
39. All existing and proposed area drains, junction boxes and utility facilities shall be raised or lowered to be flush with finished grade.
40. Locate and abandon existing sanitary laterals at the property line with the end capped and mortared. Other existing utilities shall be abandoned in accordance with the requirements of the utility owner(s).
41. When connecting new pipes to existing structures such as manholes and catch basins, the structure shall be completely cleaned out. The hole made in the structure shall be made as small as possible. The structure shall be repaired to match its original type of construction. The joint between the structure and the pipe shall be made watertight by filling the joint with mortar.
42. Flow in existing sewer system must not be interrupted. Any temporary routing of this sewer flow must be done in conformance with all applicable rules and regulations.
43. Under no circumstances shall trench water be allowed to drain off through sanitary sewer lines.
44. All crushed stone shall be Gradation No. 4 as per CT DOT Form 818, Article M.01.02. Stone shall consist of sound, tough, durable particles free from soft, thin, elongated, laminated, friable, micaceous, or disintegrated pieces of mud, dirt or other deleterious material.
45. Sanitary Sewer Testing: The sanitary sewer line shall be Low Pressure Air Tested, at the expense of the contractor; Testing to be in accordance with recommended procedure in "Unibell's" "Recommended Practice for Low Pressure Air Testing of Installed Sewer Pipe" UNI B-6. The minimum starting pressure for the test is 3.5 P.S.I. (in excess of the groundwater pressure at the top of the pipe) and there shall be no more than 0.5 P.S.I. drop in five (5) minutes. Manholes to be visually inspected. Lateral plugs shall be airtight to allow proper testing. Inspecting Engineer and the Engineering Bureau shall be informed of testing schedule three days in advance so they can witness the testing.
46. At the end of construction, after the site has be fully stabilized, all new and previously existing storm sewer facilities including, but not limited to, catch basins, area drains, manholes, junction boxes, flow control structures, pipes, oil grit separators, permeable pavers and porous pavement shall be fully cleaned with equipment designed for that purpose to the satisfaction of the inspecting engineer.

UTILITIES:

47. Utilities shown on these plans are "not guaranteed" to be complete or correct. Prior to any site activities, the contractor shall be responsible for verification of clearances of proposed utilities from existing utilities. This verification shall include physical observation by means of test pits of the locations of affected utilities. The contractor shall notify the site engineer immediately of any conflict.
48. Easements may be required in favor of the various utility companies.
49. Electric, telephone, cable, and water services shall be installed in conformance to the requirements of the governing utility companies.
50. It is the contractor's responsibility to install utilities as shown on this sheet. The contractor shall work with the utility companies and site engineer to insure the installation is in conformance to the requirements of the governing utility company. All conduits shall be concrete encased as may be required by the governing utility company. Proposed electric, telephone, cable and water services are shown for schematic purposes only and are subject to change pending utility company review. These utilities shall be designed by others and installed in conformance to the requirements of the governing utility companies.
51. All proposed utility facilities shall be raised or lowered to be flush with finished grade.
52. Where necessary, existing utilities shall be reinstalled to meet all minimum coverage requirements.
53. Utility connections at building face shall be coordinated with the building contractors.
54. The contractor must supply and install drag lines with all conduits.
55. Assume one 2" PVCP conduit for all site lighting. Service location to be determined.
56. In general, each utility shall have a minimum clearance of three feet to any other underground utility.
57. Any and all utilities abandoned shall be capped or removed in accordance with utility companies' requirements.
58. Existing fire valves shall be cut flush to grade in accordance with Aquarion Water Company requirements.
59. The electric transformer and generator shall be located to meet all applicable Zoning setbacks.
60. Detectable Tape shall be used to mark piping listed below. The identification tape shall be buried at least 6-inches to 10-inches below final grade but no closer than 12-inches to the buried utility piping or service.

Electric	Red	Caution Electric Line Buried Below
Telephone & Control	Orange	Caution Telephone Line Buried Below
Natural Gas	Yellow	Caution Gas Line Buried Below
Water Systems	Blue	Caution Water Line Buried Below
Fire Protection Systems	Blue	Caution Fire Line Buried Below Sprinkler
Mains	Blue	Caution Sanitary Line Buried Below Sewer
System	Green	Caution Sewer Line Buried Below
IS & S Communication Conduit	Orange	Conc. N/A

61. Underground-Type Plastic Line Marker: Manufacturer's standard permanent, bright-colored detectable tape, continuous-printed plastic tape, intended for direct-burial service; not less than 6" wide X 4 mils thick.

PAVEMENT AND PAVEMENT MARKINGS:

62. Areas of asphalt pavement that are disturbed by the construction of this project shall be replaced in accordance with the asphalt pavement repair detail. The finished grade of asphalt paving shall blend to existing grade and the edge of the concrete pavement smoothly with no slopes exceeding 4%.
63. Existing features such as but not limited to walks, curbs, and pavement damaged by construction activities shall be repaired at no additional cost to the owner.
64. Saw cut perimeter of area to be excavated. Saw cut shall be straight and vertical.
65. Contractor shall engage a testing lab who shall verify the base course material by means of a sieve analysis and perform compaction testing of the base and each course of pavement. Site Engineer shall review with the contractor the required testing at the preconstruction meeting. Site Engineer shall approve base course prior to placement of each layer of pavement.
66. The Contractor shall engage a qualified independent testing agency to perform field inspections and tests and to prepare test reports. Testing agency will conduct and interpret tests and state in each report whether tested work complies with or deviates from specified requirements.
67. Additional testing, at Contractor's expense, will be performed to determine compliance of corrected work with specified requirements. Remove and replace or install additional hot-mix asphalt where test results or measurements indicate that it does not comply with specified requirements as directed by the Site Engineer.
68. Contractor is responsible to place the hot-mix asphalt mix as required in the drawings, details and the applicable Section of the CT DOT FORM 818 (latest edition).
69. Compaction shall be constructed as specified in the CT DOT FORM 818 (latest edition), Section 4.06 specification, the drawings and the details. Testing lab shall verify compaction of each course of pavement as directed by the Site Engineer.
70. After the asphalt pavement has cured sufficiently to support the weight of a water truck without marking the newly installed pavement, is shall be water tested for low spots, areas of little or no drainage, etc. A water truck shall spray a sufficient amount of water on all pavement sections to observe the drainage of water. There shall be positive drainage on all areas of the pavement. Any visible low spots where significant water (greater than or equal to 3/16" in depth) is left standing, shall be clearly marked for the Contractor to repair prior to final acceptance. These areas must be sawcut and removed down to the base course prior to replacement with asphalt mixture as per the original approved design. The base course and edges of sawcut asphalt must be treated with tack oil prior to new section of asphalt being installed. The Owner's Representative or inspecting A/E shall be notified 48 hours in advance of water test so that he may be present during the test.
71. The inspecting engineer and contractor will review the testing requirements at the preconstruction meeting. At this meeting, samples to be tested and compaction testing protocol will be discussed. Testing and approval of the subgrade, base course and asphalt layers prior to the installation of the next layer to determine if the work complies or deviates from the specified requirements. Prior to installation of the base course, contractor shall contact inspecting engineer to determine the suitability of the subgrade material, base course and asphalt. Additional excavation or base course may be required.
72. Finished paving shall be free of ``bird baths" and be smooth at the slopes specified on the plans.
73. Finished grade shall be within 1/2 inch of that noted on the drawings.
74. The pavement shall be protected from vehicular traffic of any kind with the use of barricades, etc. for a minimum period of 24 hours after final rolling. Maintain and protect asphalt surface from scrapes, sears, spills, hydraulic leaks, and any other construction damage for the remainder of construction until Owner's Representative acceptance. Contractor is responsible for clearing, repairing, seal coating, patching, and re-striping as necessary to obtain Owner's Representative's final approval/acceptance.
75. Thicknesses of all layers shown are after compaction. Compact all layers to 95% per ASTM D 1557 (Modified Proctor Method).
76. All pavement striping and replacement shall conform to the Town of Wilton standards and the latest edition of AASHTO Highway Design Manual.

DPW CONDITIONS:

77. Easements shall be created portions of roadway and sidewalks providing pedestrians access that fall on the subject property.
78. Prior to construction brick samples along sidewalks shall be provided to match existing bricks.
79. Final design plans shall be submitted to DPW for review prior to the issuance of a Building Permit. The sidewalk details, re-use of existing light pole foundations, and pavement restoration limits shall be reviewed and finalized as part of the Building Permit application.
80. Prior to the issuance of a Certificate of Occupancy, a certified as-built drawing and certified letter signed by a Professional Engineer indicating that all work was completed in accordance with the design plans shall be submitted to the Town of Wilton.
81. The proposed stormwater system is connecting directly to the roadway drainage system. Any damage to the proposed development caused by stormwater back up due to a clogged catch basin or insufficient pipe capacity shall not be the responsibility of the Town.
82. Prior to any work in the Town Right of Way, a Road Opening Permit shall be obtained.
83. The project is subject to obtaining approvals from Wilton's WPCA Commission to connect additional units into the sanitary sewer system.
84. Project is subject to Norwalk WPCA's review and comment.
85. The project will be subject to Sewer Capital Assessment as required by the WPCA.
86. No footing drains or sumps shall connect to the sanitary system.
87. Property owner shall be responsible for maintenance of the lateral and unclogging any potential clogs in the lateral and/or sewer main connection points.
88. All proposed sewer lines shall be air tests prior to sign off of the Certificate of Occupancy.
89. The project is subject to the final technical review by WPCA.

No.	Date	Revision
6	05/15/2023	REVISED PER PZC COMMENTS
5	05/08/2023	REVISED PER DPW COMMENTS
4	02/28/2023	REVISED PER BUILDING DESIGN
3	01/02/2023	REVISED PER DPW COMMENTS
2	10/20/2022	REVISED PER FIRE MARSHALL'S COMMENTS
1	09/30/2022	ORIGINAL ISSUE DATE

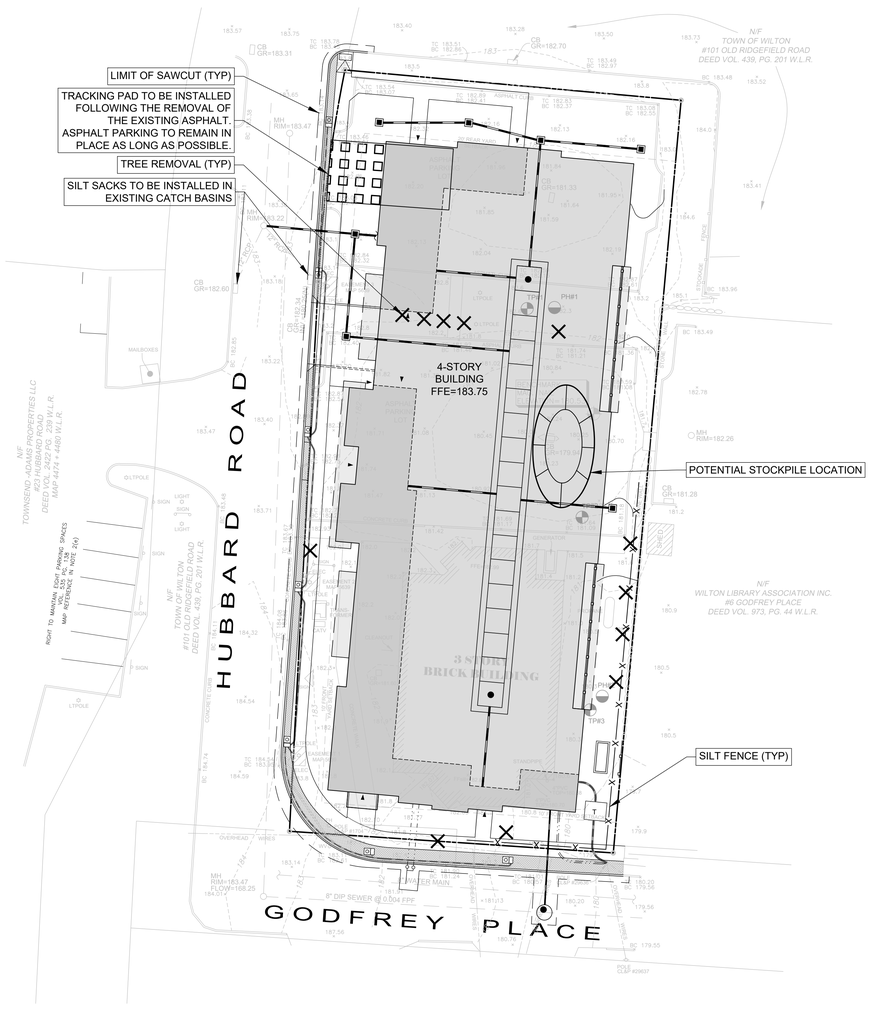
SITE DEVELOPMENT PLAN
DEPICTING
12 GODFREY PLACE
WILTON, CT
PREPARED FOR
GREENWICH REALTY DEVELOPMENT, LLC

REDNISS & MEAD
LAND SURVEYING
CIVIL ENGINEERING
PLANNING & ZONING CONSULTING
PERMITTING
22 First Street | Stamford, CT 06905
Tel: 203.327.0500 | Fax: 203.357.1118
www.rednissmead.com

SCALE: 0 20 40
1"=20'
DRAWN BY: PBS | CHECKED BY: CJF

CRAIG J. FLAHERTY CT. P.E. 21149
May 15, 2023
DATE

SHEET No:
SE-1
Comm. No.: 10556



SEDIMENT AND EROSION CONTROL NARRATIVE:

The purpose of the Sediment and Erosion Control Plan, details, and notes is to outline a program that minimizes soil erosion during construction. The primary policies of this program are:

a) Trapping particles at source by promptly stabilizing disturbed areas;
b) Avoid concentration of water;
c) Avoid contamination of existing storm drains;
d) Maintenance (weekly maintenance and after storm events) of controls to ensure they are functioning properly;

SEDIMENT AND EROSION CONTROL NOTES:

1. Sheet SE-2 is intended to describe the soil sediment and erosion control treatment of this site only. For other details with respect to construction, see appropriate drawings.
2. All sediment and erosion controls shall be done in conformance with the "Connecticut Guidelines for Soil Erosion and Sediment Control" dated May 2002 prepared by The Connecticut Council on Soil and Water Conservation.
3. The contractor is assigned the responsibility for implementing this sediment and erosion control plan. This responsibility includes the installation and maintenance of control measures, informing all parties engaged on the construction site of the requirements and objectives of the plan notifying the Zoning Department of any transfer of this responsibility and that construction is to begin three (3) days prior to commencing work.
4. Temporary sediment control measures must be installed in accordance with drawings and manufacturer recommendations prior to work in any upland areas.
5. No construction or construction equipment or storage of materials will be allowed on the downhill side of the silt fence or within fenced off areas, except during construction of the proposed facilities shown beyond the fences.
6. Anti-tracking pads shall be installed at start of construction and maintained in an effective condition throughout the duration of construction. Pads consist of 2" - 4" crushed stone, 6" minimum thickness and extend the width of the construction access. The length of the access shall be sufficient to prevent dirt from being tracked onto off site roads (minimum length of 50').
7. The location of each stockpile will vary throughout the construction period. Excavated silt and earth stockpiles shall be stored on site. Silt fence shall be placed at the base of the stockpile to prevent sediment from leaving the site and to protect storm drains, wetlands and watercourses.
8. Silt fence shall be Mirafi envirofence, Amoco siltstop or equivalent approved by Site Engineer. Filter fabric used shall be Mirafi 100x or equivalent. Install silt fence according to manufacturer's instruction, particularly, bury lower edge of fabric into ground.
9. Land disturbance shall be kept to a minimum. All disturbed area shall be planted in where permanent plantings are called for as soon as practicable. Seed and mulch disturbed areas with grass seed where permanent plantings are not called for, as soon as practicable. Prepare seedbed (4" thick minimum) with topsoil. Seed, rake, roll, water and mulch areas according to mixes below. Water as often as necessary (up to 3 times per day) to establish cover. Mulch seeded areas at 1 to 2 tons/acre with salt hay. Maintain mulch and watering until grass is 3" high with 85% cover. Reseed or overseed if necessary.

   Temporary Seed Mix:
   Perennial ryegrass — 40 lbs/ac. (1 lb/1000 sf.)

   Permanent Lawns:
   Kentucky Bluegrass — 20 lbs/ac.
   Creeping Red Fescue — 20 lbs/ac.
   Perennial Ryegrass — 5 lbs/ac.
   45 lbs/ac. (1 lb/1000 sf.)

   Optimum Seeding Dates:
   April 15 through June 15
   August 15 through October 1
10. Any disturbed area shall be restored to the preconstruction condition. Existing shrubs shall be carefully dug up, stored in a temporary nursery during the project and replanted as directed by the Owner. The time during which these bushes are out of the ground must be minimized. The contractor shall keep the shrubs watered and out of the direct sun during this time.
11. If disturbed areas can not be seeded immediately due to the time of year, mulch area until seeding can occur; remove mulch and seed and remulch when season permits.
12. Upon installation of each catch basin and area drain, immediately surround it with haybales as per sediment filter detail.
13. Haybales shall be new and are to be replaced whenever their condition deteriorates beyond reasonable usability.
14. Temporarily block pipes leading into the storm water infiltration system until upland areas are thoroughly stabilized. Under no circumstances shall sediment or silty water be allowed to enter the infiltration system.
15. Pavement and curbing should be placed as soon as possible after drainage is installed.
16. Loaded trucks shall be covered as required to keep down dust.
17. Affected portions of off site roads and sidewalks must be swept clean when required to keep down dust and prevent safety hazards or at least once a week during construction and as directed by Site Engineer.
18. Dust control to be achieved with watering down disturbed areas as required.
19. All sediment and erosion controls shall be inspected periodically throughout construction. Any corrective actions to mitigate environmental concerns will be ordered by the site engineer or environmental engineer. It is the Owner's responsibility to retain such consultant.
20. Additional sediment and erosion control measures may be installed during the construction period if found necessary by the inspecting engineer or any Governing Agency.
21. All permanent and temporary sediment control devices will be maintained in effective condition throughout the construction period until upland disturbed areas are thoroughly stabilized. Upon completion of work and stabilization of all upland areas, all temporary sediment control devices and tree protection should be removed from the site and any silt disposed of legally.
22. Excavated silt and earth stockpiles shall not be permitted to be stored on site. Excess material shall be disposed of legally.
23. Periodically and upon completion of the job, clean silt from any effected storm sewer systems including pipes and inlets. Use silt during final landscaping or dispose off-site legally.

CONSTRUCTION PHASING:

The following description of construction phasing is intended to demonstrate a feasible sequence of construction. The actual sequence may vary due to field conditions if approved by the inspecting engineer.

PHASE I: PREPARATION

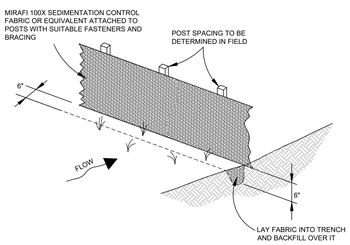
A. AT LEAST ONE WEEK PRIOR TO THE START OF CONSTRUCTION, THE INSPECTING ENGINEER SHALL MEET WITH THE CONTRACTOR AND OWNER TO REVIEW THE SEDIMENT AND EROSION CONTROL (S&E PLAN), DISCUSS ANY MODIFICATIONS TO CONSTRUCTION SEQUENCE OR S&E PLAN AND TO REVIEW CONTRACTORS LOGISTICS PLAN.
B. ESTABLISH STAGING AREA WITH TRAILERS AND TEMPORARY UTILITIES.
C. INSTALL TRACKING PADS FOR CONSTRUCTION ACCESS.
D. INSTALL SILT FENCE, CONSTRUCTION FENCE AND PERIMETER FENCE AS SHOWN ON THE PLANS.
E. CUT TREES TO BE REMOVED AND GRUB AREAS TO BE CLEARED.
F. REMOVE/DEMOLISH EXISTING BUILDING. REMOVE EXISTING PAVEMENT ONLY AS NECESSARY TO PROCEED WITH EACH PHASE OF CONSTRUCTION.

PHASE II: CONSTRUCTION

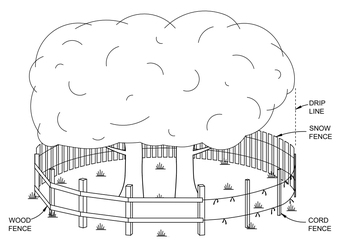
A. ROUGH GRADE SITE. GENERAL EARTHWORK. EXCAVATE FOR BUILDING FOUNDATION. INSTALL CONSTRUCTION DEWATERING AND TEMPORARY FILTERING SYSTEM AS NECESSARY. COORDINATE DEWATERING CONSTRUCTION WITH SITE GEOTECHNICAL AND STRUCTURAL ENGINEERS. (NOTE: MANAGEMENT OF EXCAVATED MATERIALS DURING THIS PROCESS SHALL BE ACHIEVED BY TEMPORARILY STOCKPILING ONSITE TO THE EXTENT CONSTRUCTION STAGING WILL ALLOW AND BY HAULING MATERIAL OFFSITE AS EXCAVATED).
B. CONSTRUCT FOUNDATION AND BACKFILL AS SOON AS POSSIBLE.
C. INSTALL STORM WATER SYSTEM. THE DRAINAGE UTILITIES WILL BE INSTALLED AND READY TO RECEIVE STORM WATER PRIOR TO THE INSTALLATION OF PAVING.
D. INSTALL SEDIMENT AND EROSION CONTROLS ASSOCIATED WITH DRAINAGE STRUCTURES.
E. INSTALL SANITARY, WATER, CABLE, ELECTRIC, AND TELEPHONE UTILITIES.
F. FINAL GRADING AND PAVING.
G. SEED & MULCH DISTURBED AREAS AND INSTALL LANDSCAPING AS SOON AS POSSIBLE.
H. MAINTAIN ALL SEDIMENT AND EROSION CONTROLS IN AN EFFECTIVE CONDITION DURING THE CONSTRUCTION PERIOD.

PHASE III: CLEAN UP AFTER ALL AREAS ARE STABILIZED

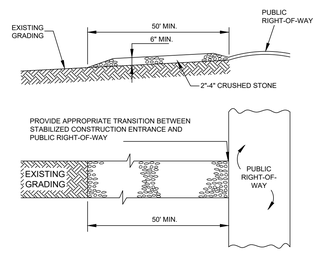
A. CLEAN EFFECTED PORTION OF ON & OFF SITE ROADS AND DRIVEWAYS.
B. REMOVE ACCUMULATED SILT AND DEBRIS FROM CATCH BASIN SUMPS & PIPES OF EFFECTED ON & OFF SITE STORM DRAINS.
C. REMOVE ACCUMULATED SEDIMENT FROM EFFECTED AREAS AND DISPOSE OF LEGALLY.
D. REMOVE TEMPORARY SEDIMENT AND EROSION CONTROL AND TREE PROTECTION.
E. MAKE ANY NECESSARY REPAIRS TO PERMANENT SEDIMENT AND EROSION CONTROLS SUCH AS PLANTINGS.



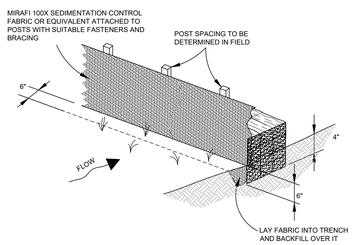
FABRIC & POST SILTATION BARRIER
(SILT FENCE)
N.T.S.



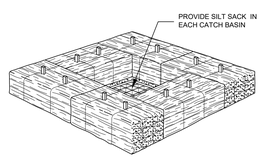
TREE PROTECTION
(SHOWING ACCEPTABLE TYPES OF FENCING)
N.T.S.



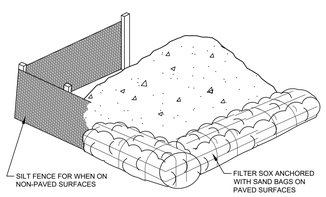
STABILIZED CONSTRUCTION ENTRANCE
(TRACKING PAD)
N.T.S.



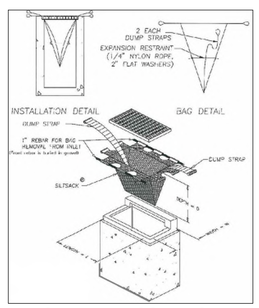
FABRIC & POST SILTATION BARRIER W/ HAY BALES
(SILT FENCE)
N.T.S.



SEDIMENT FILTER FOR CATCH BASINS
N.T.S.



SEDIMENT FILTER FOR STOCK PILE
N.T.S.



INLET SEDIMENT CONTROL DEVICE
(SILT SACK)
N.T.S.

No.	Date	Revision
4	05/08/2023	REVISED PER DPW COMMENTS
3	02/28/2023	REVISED PER BUILDING DESIGN
2	01/02/2023	REVISED PER DPW COMMENTS
1	09/30/2022	ORIGINAL ISSUE DATE

SEDIMENTATION & EROSION CONTROL PLAN

DEPICTING

12 GODFREY PLACE

WILTON, CT

PREPARED FOR

GREENWICH REALTY DEVELOPMENT, LLC

SCALE: 1"=20' (0, 20, 40)

DRAWN BY: PBS | CHECKED BY: CJF

CRAIG J. FLAHERTY CT. P.E. 21769

May 8, 2023

DATE

REDNISS & MEAD
LAND SURVEYING
CIVIL ENGINEERING
PLANNING & ZONING CONSULTING
PERMITTING
22 First Street | Stamford, CT 06905
Tel: 203.327.0500 | Fax: 203.357.1118
www.rednissmead.com

SHEET No: SE-2

Comm. No.: 10556

NORTH — MAP 39/0 W.L.R.

TEST PIT DATA

Subsurface Soil Investigation Soil Profile	
Test Pit #: 1	Date: 08/24/2022
Inspector: PBS	Sanitarian: N/A
Ledge at: N/A	Mottling at: N/A
Water at: N/A	Roots at: 62"
Depth: 70"	Soil Description
0"-4"	Top Soil
4"-70"	Light Brown Silty Sand w/ gravel and cobbes (Bank Run Gravel)
Subsurface Soil Investigation Soil Profile	
Test Pit #: 2	Date: 08/24/2022
Inspector: PBS	Sanitarian: N/A
Ledge at: N/A	Mottling at: N/A
Water at: N/A	Roots at: N/A
Depth: 66"	Soil Description
0"-4"	Asphalt
4"-12"	Processed Road Base
12"-66"	Light Brown Silty Sand w/ gravel and cobbes (Bank Run Gravel)
Subsurface Soil Investigation Soil Profile	
Test Pit #: 3	Date: 08/24/2022
Inspector: PBS	Sanitarian: N/A
Ledge at: N/A	Mottling at: N/A
Water at: N/A	Roots at: 50"
Depth: 61"	Soil Description
0"-6"	Top Soil
6"-61"	Light Brown Silty Sand w/ gravel and cobbes (Bank Run Gravel)

TEST PIT DATA

Recorded By: PBS	Date: 08/24/22
Hole: 1	Project: 10556
Depth: 24"	Diameter: 8"
9:00 AM	1:02 hrs

Minimum Uniform Drop: 6/16 inches in 5 minutes

Percolation Rate = 1"drop in 13.33 minutes

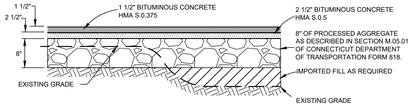
Time	Reading In Inches Total	Increment Drop In Inches
10:02 AM	5 4/16	-
10:07 AM	6 7/16	1 3/16
10:12 AM	7 6/16	15/16
10:17 AM	8 4/16	14/16
10:22 AM	8 12/16	8/16
10:27 AM	9 2/16	6/16
10:32 AM	9 8/16	6/16
10:37 AM	9 14/16	6/16
10:42 AM	10 5/16	7/16
10:47 AM	10 11/16	6/16
10:52 AM	11 1/16	6/16
10:57 AM	11 7/16	6/16
11:02 AM	11 13/16	6/16

Recorded By: PBS	Date: 08/24/22
Hole: 2	Project: 10556
Depth: 24"	Diameter: 8"
9:00 AM	1:00 hrs

Minimum Uniform Drop: 8/16 inches in 5 minutes

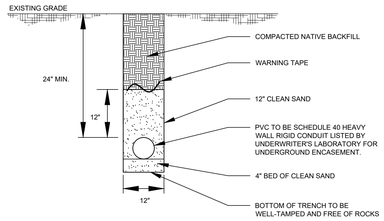
Percolation Rate = 1"drop in 10.00 minutes

Time	Reading In Inches Total	Increment Drop In Inches
10:00 AM	9 4/16	-
10:05 AM	10 10/16	1 6/16
10:10 AM	12	1 6/16
10:15 AM	12 12/16	12/16
10:20 AM	13 10/16	14/16
10:25 AM	14 6/16	12/16
10:30 AM	14 14/16	8/16
	Fill	
10:30 AM	10 10/16	10 10/16
10:35 AM	11 13/16	1 3/16
10:40 AM	12 10/16	13/16
10:45 AM	13 2/16	8/16
10:50 AM	13 11/16	9/16
10:55 AM	14 3/16	8/16
11:00 AM	14 11/16	8/16



PAVEMENT DETAIL

N.T.S.

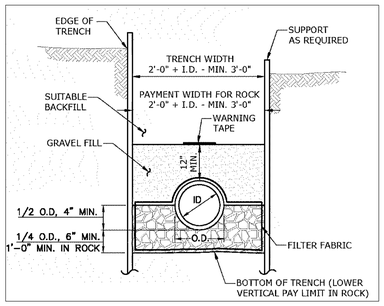


NOTES:

1. IF 24" OF COVER CANNOT BE OBTAINED OVER THE CONDUIT, CONDUIT SHALL BE CONCRETE ENCASED.
2. ALL BACKFILL MATERIAL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D1557.

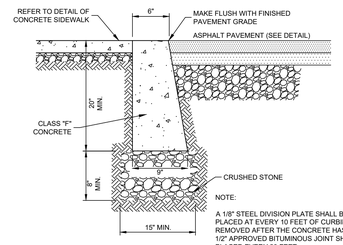
LIGHTING CONDUIT TRENCH DETAIL (SAND BEDDING)

N.T.S.



SANITARY SEWER TRENCH DETAIL

N.T.S.

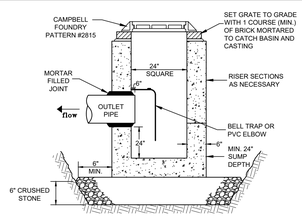


NOTE:

A 1/8" STEEL DIVISION PLATE SHALL BE PLACED AT EVERY 10 FEET OF CURBING AND REMOVED AFTER THE CONCRETE HAS SET. A 1/2" APPROVED BITUMINOUS JOINT SHALL BE PLACED EVERY 20 FEET.

FLUSH CONCRETE CURB

N.T.S.



NOTES:

1. ALL CATCH BASIN COMPONENTS TO BE PRE-CAST REINFORCED CONCRETE, ABLE TO WITHSTAND THE APPLIED EARTH LOADS WITH AN H-20 TRUCK LOAD.
2. ALL JOINTS TO BE MORTARED.
3. AREA DRAIN SHALL CONFORM TO ASTM C478.
4. ALL CRUSHED STONE SHALL BE GRADATION NO. 4 AS PER CT D.O.T. FORM 818, ARTICLE M.01.01. STONE SHALL CONSIST OF SOUND, TOUGH, DURABLE PARTICLES FREE FROM SOFT, THIN, ELONGATED, LAMINATED, FRIABLE, MICACEOUS OR DISINTEGRATED PIECES, MUD, DIRT OR OTHER DELETERIOUS MATERIAL.

24" AREA DRAIN

N.T.S.

NOTES:

1. ALL COMPONENTS TO BE PRE-CAST REINFORCED CONCRETE, ABLE TO WITHSTAND THE APPLIED EARTH LOADS OF AN H-20 TRUCK LOAD.
2. ALL JOINTS TO BE MORTARED.
3. JUNCTION BOXES SHALL CONFORM TO ASTM C478.
4. ALL CRUSHED STONE SHALL BE GRADATION NO. 4 AS PER CT D.O.T. FORM 818, ARTICLE M.01.01. STONE SHALL CONSIST OF SOUND, TOUGH, DURABLE PARTICLES FREE FROM SOFT, THIN, ELONGATED, LAMINATED, FRIABLE, MICACEOUS OR DISINTEGRATED PIECES, MUD, DIRT OR OTHER DELETERIOUS MATERIAL.

JUNCTION BOX

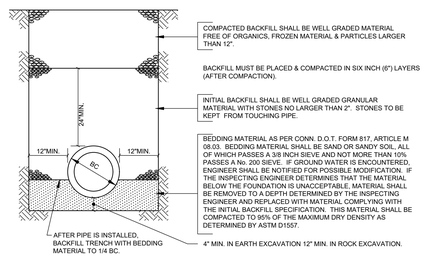
N.T.S.

WATER STOP: 10' UPSTREAM OF STRUCTURES AND WHERE SHOWN, FOUNDATION MATERIAL, BEDDING, HAUNCHING, INITIAL BACKFILL, AND THE BOTTOM FOOT OF GENERAL BACKFILL TO BE REPLACED WITH SM, SC, OR ML SOIL AS PER "UNIFIED SOIL CLASSIFICATION SYSTEM" WITH MAXIMUM PARTICLE SIZE OF 1-1/2", FOR 3 LINEAR FEET OF TRENCH. WATER STOP TO BE KEYED INTO TRENCH BOTTOM AND WALLS A MINIMUM OF ONE FOOT. NO STONES LARGER THAN 6" SHALL BE WITHIN 12" OF THE PIPE.

ALL FOUNDATION, INITIAL BACKFILL & BACKFILL MATERIAL TO BE APPROVED BY THE INSPECTING ENGINEER.

ANY DEVIATION FROM THESE METHODS & MATERIALS MUST BE APPROVED IN WRITING BY THE INSPECTING ENGINEER.

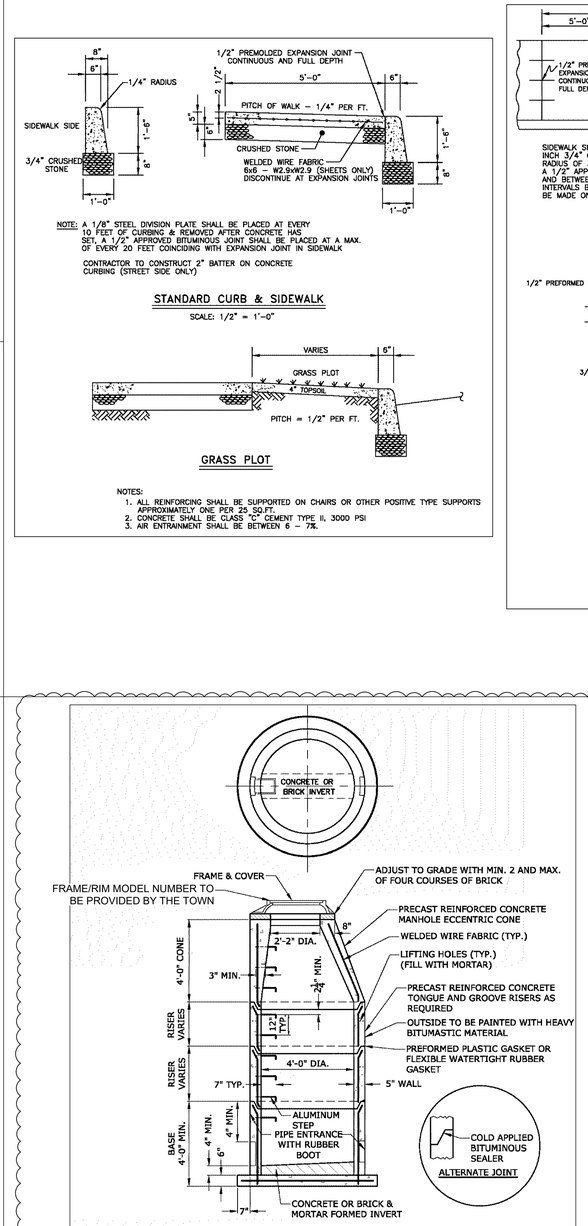
ALL MATERIAL TO BE COMPACTED TO 95% OF THE MAX. DRY DENSITY AS DETERMINED BY ASTM D1557, EXCEPT COMPACTED BACKFILL" NOT UNDER PAVEMENT WHICH SHALL BE COMPACTED TO A DENSITY AT LEAST EQUAL TO THAT OF THE ADJACENT UNDISTURBED MATERIAL.



PVC/RCP PIPE TRENCH BEDDING DETAIL

(48" DIA. & UNDER)

N.T.S.



NOTE: A 1/8" STEEL DIVISION PLATE SHALL BE PLACED AT EVERY 10 FEET OF CURBING & REMOVED AFTER CONCRETE HAS SET, A 1/2" APPROVED BITUMINOUS JOINT SHALL BE PLACED AT A MAX. OF EVERY 20 FEET COINCIDING WITH EXPANSION JOINT IN SIDEWALK

CONTRACTOR TO CONSTRUCT 2" BATTER ON CONCRETE CURBING (STREET SIDE ONLY)

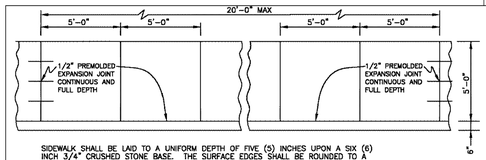
STANDARD CURB & SIDEWALK

SCALE: 1/2" = 1'-0"

GRASS PLOT

NOTES:

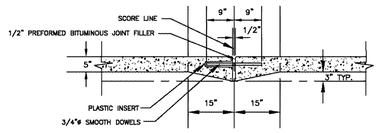
1. ALL REINFORCING SHALL BE SUPPORTED ON CHAIRS OR OTHER POSITIVE TYPE SUPPORTS
2. CONCRETE SHALL BE CLASS "C" CEMENT TYPE II, 3000 PSI
3. AIR ENTRAINMENT SHALL BE BETWEEN 6 - 7%.



SIDEWALK SHALL BE LAID TO A UNIFORM DEPTH OF FIVE (5) INCHES UPON A SIX (6) INCH 3/4" CRUSHED STONE BASE. THE SURFACE EDGES SHALL BE ROUNDED TO A RADIUS OF 1/2 INCH.
A 1/2" APPROVED BITUMINOUS JOINT SHALL BE PLACED TRANSVERSELY EVERY 20 FT. MAX. AND BETWEEN NEW CONCRETE CURBING AND SIDEWALKS. A CONTROL JOINT SHALL BE MADE AT 5' INTERVALS BETWEEN BITUMINOUS JOINTS. ANY CHANGES REQUIRED BY LOCAL CONDITIONS SHALL BE MADE ONLY BY ORDER OF THE CITY ENGINEER. CONTROL JOINTS SHALL BE A MIN. DEPTH OF 1" OR D/4

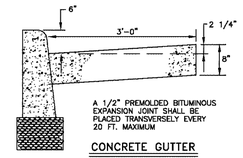
PLAN OF A SECTION OF CONCRETE SIDEWALK

SCALE: 1/4" = 1'-0"



PREMOULDED EXPANSION JOINT DETAIL

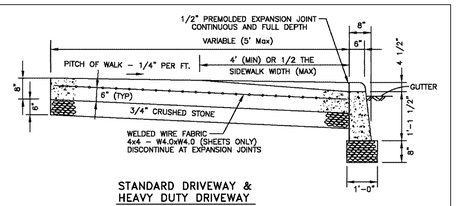
SCALE: 1/2" = 1'-0"



A 1/2" PREMOLDED BITUMINOUS EXPANSION JOINT SHALL BE PLACED TRANSVERSELY EVERY 20 FT. MAXIMUM

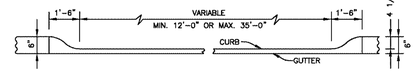
CONCRETE GUTTER

SCALE: 3/4" = 1'-0"



STANDARD DRIVEWAY & HEAVY DUTY DRIVEWAY

SCALE: 1/2" = 1'-0"



TYPICAL CONSTRUCTION OF CURB AT DRIVEWAY

SCALE: 1/2" = 1'-0"

GENERAL NOTES:

1. ALL REINFORCING SHALL BE SUPPORTED ON CHAIRS OR OTHER POSITIVE TYPE SUPPORTS APPROXIMATELY ONE PER 25 SQ.FT.
2. CONCRETE SHALL BE CLASS "C" CEMENT TYPE II, 3000 PSI
3. AIR ENTRAINMENT SHALL BE BETWEEN 6 - 7%.
4. A 1/2" PREMOLDED EXPANSION JOINT SHALL BE UTILIZED BETWEEN ALL RIGID STRUCTURES INCLUDING WALLS AND NEW SIDEWALK WORK.
5. ADDITIONAL CONTROL JOINTS SHALL BE PLACED AS REQUIRED TO ELIMINATE ANY CONDITION WHICH WILL CAUSE STRESS VERTICES (EXAMPLE AT CORNERS OF STRUCTURES)

SIDEWALK & CURBING DETAILS

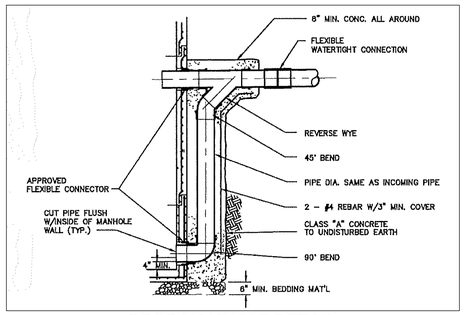
N.T.S.

5' OR 6' DIA. PRECAST BASES MAY BE USED WHEN REQUIRED DUE TO SIZE OR NUMBER OF PIPES AT THE MANHOLE.
PRECAST REDUCERS WILL BE PLACED ABOVE THE 5' & 6' BASES AS DIRECTED BY THE ENGINEER.
WALL THICKNESS TO INCREASE 1" FOR EACH 1' OF INSIDE DIAMETER INCREASE.

NOTE: THE TOWN HAS AN UPDATED VERSION OF THE SANITARY SEWER MANHOLE FRAME AND COVER. THE UPDATED MODEL NUMBER WILL BE PROVIDED AS PART OF THE BUILDING PERMIT REVIEW.

PRECAST SANITARY DETAIL

N.T.S.



DROP MANHOLE DETAIL

N.T.S.

NOTE:

1. ASPHALT THICKNESS AS PART OF FINAL RESTORATION SHALL MATCH THE THICKNESS OF THE EXISTING ROAD OR THE MINIMUM OF 4", WHICHEVER IS GREATER.
2. FINAL PAVEMENT LIMITS SHALL BE AS APPROVED BY TOWN ENGINEER.
3. ALL EXCAVATION SHALL BE PATCHED WITH TEMPORARY BITUMINOUS CONCRETE DAILY.
4. TEMPORARY ASPHALT SHALL BE 4" INCHES THICK, INSTALLED IN 2 LIFTS.

ASPHALT TRENCH REPAIR

N.T.S.

NO.	DATE	DESCIPTION	CK.
4	05/08/2023	REVISED PER DPW COMMENTS	
3	02/28/2023	REVISED PER BUILDING DESIGN	
2	01/02/2023	REVISED PER DPW COMMENTS	
1	09/30/2022	ORIGINAL ISSUE DATE	
No.	Date	Revision	

REVISIONS

TOWN OF WILTON
Department of Public Works
SKETCHES & CALCULATIONS

Project No. & Name STANDARDS
Subject SIDEWALKS & CURBING
Made by F. Smeriglio, PE Date 2/20/19
Chk. by F. Smeriglio, PE Date 2/20/19
SW-2 Sheet No. 2 of 2

DETAILS & SOIL DATA
DEPICTING
12 GODFREY PLACE
WILTON, CT
PREPARED FOR
GREENWICH REALTY DEVELOPMENT, LLC

SCALE:N.T.S.

DRAWN BY: PBS | CHECKED BY: CJF

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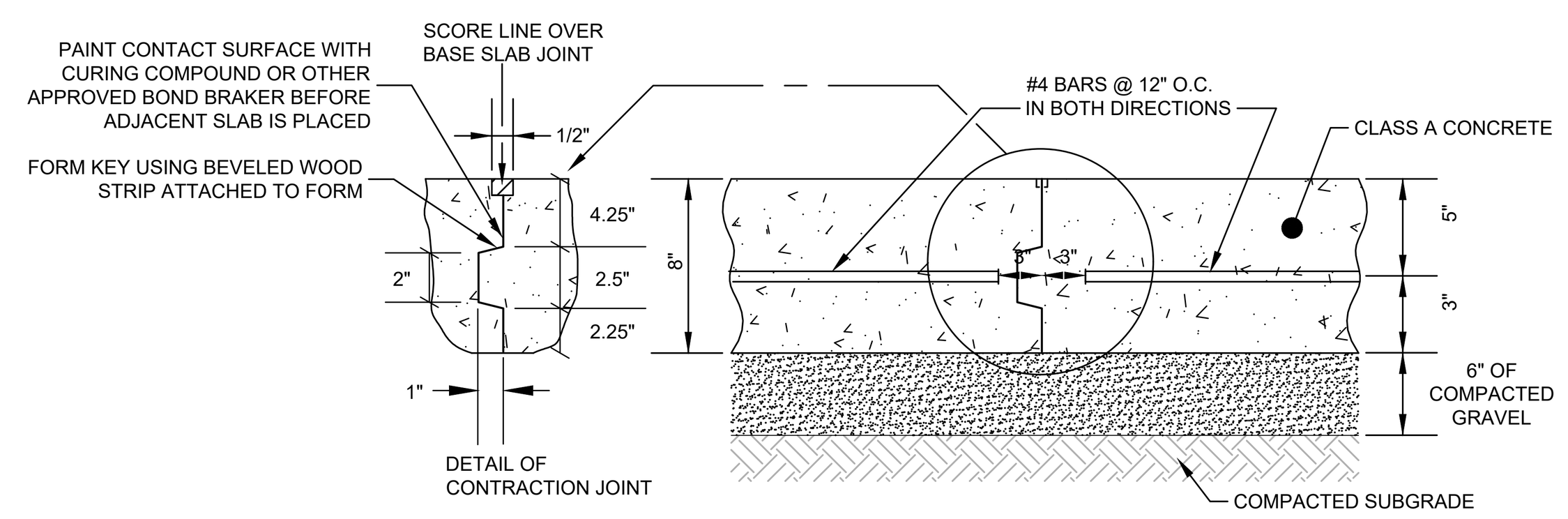
CRAIG J. FLAHERTY CT. P.E. 21149
May 8, 2023
DATE

SHEET No:

SE-3

Comm. No.: 10556



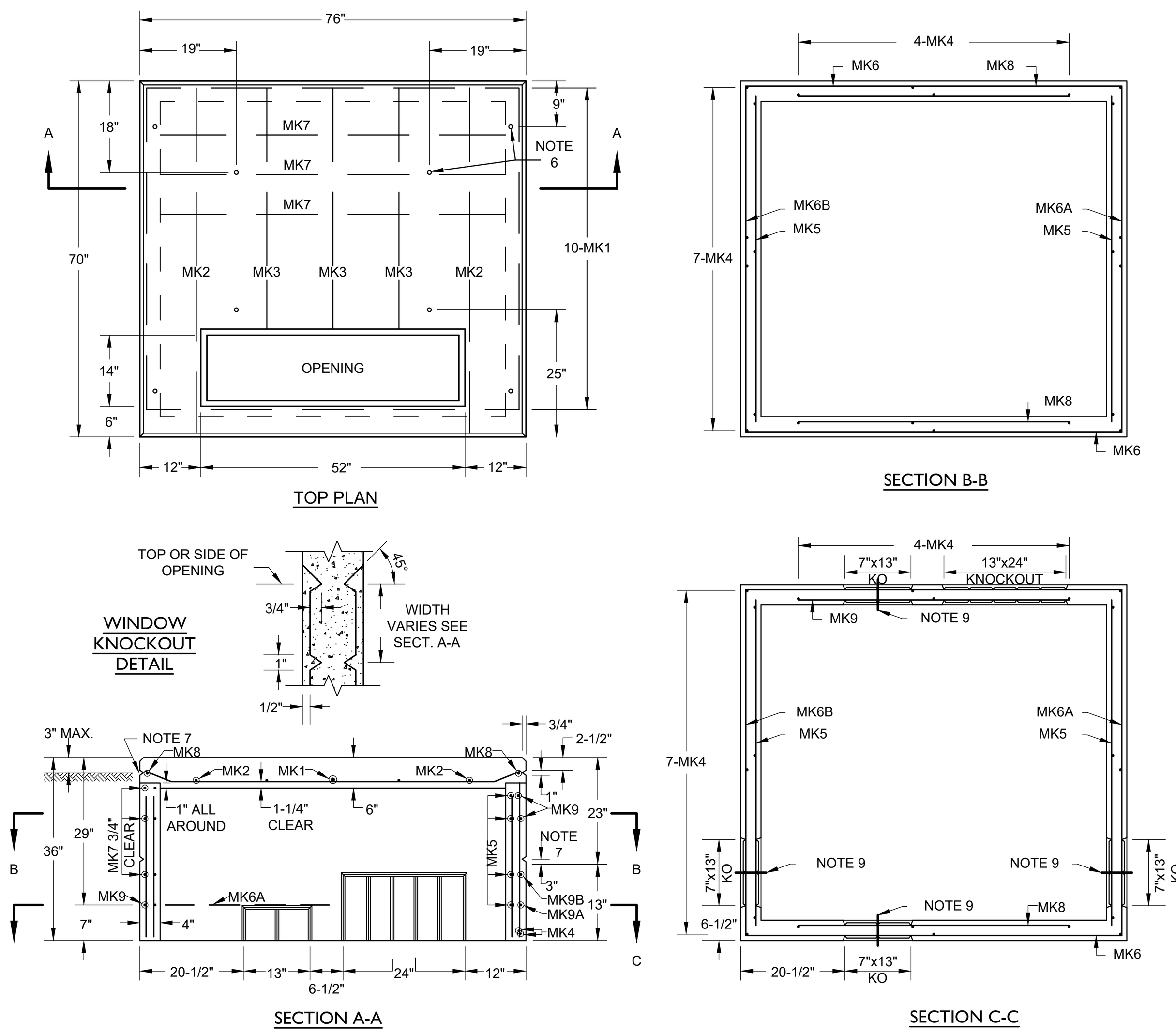
NOTES:

1. CONCRETE TO BE CLASS 'A' CONFORMING TO CONNDOT FORM 817 SECTION M.03.02.
2. GRAVEL BASE SHALL CONFORM TO GRADATION A AS DEFINED IN CONNDOT FORM 817 SECTION M. 02.01.
3. INSTALL AS PER THE AMERICAN CONCRETE INSTITUTE CODE.
4. THE AREA SHALL BE COMPACTED TO AT LEAST 95% OF THE DRY DENSITY ACHIEVED BY AASHTO T180, METHOD D.
5. CONTRACTION JOINT TO BE PLACED SO REMAINING SECTIONS OF CONCRETE ARE GENERALLY SQUARE OR AT LEAST EVERY 10'.
6. EDGES OF CONCRETE TO BE TOOLED TO A 1/2" RADIUS.
7. SIZE OF PAD TO BE VERIFIED PRIOR TO CONSTRUCTION.

CONCRETE PAD DETAIL

N.T.S.

BAR SCHEDULE											
MARK NO.	MK1	MK2	MK3	MK4	MK5	MK6	MK6A	MK6B	MK7	MK8	MK9
SIZE	#6	#4	#4	#3	#4	#4	#4	#4	#4	#4	#4
NO. OF BARS	10	2	3	22	8	7	1	1	2	7	1
DIMENSIONS	62" 5" 2"	56" 5" 2"	41" 5" 2"	29"	67"	26" 74" 26"	74" 26"	10" 38"	68"	56"	29"

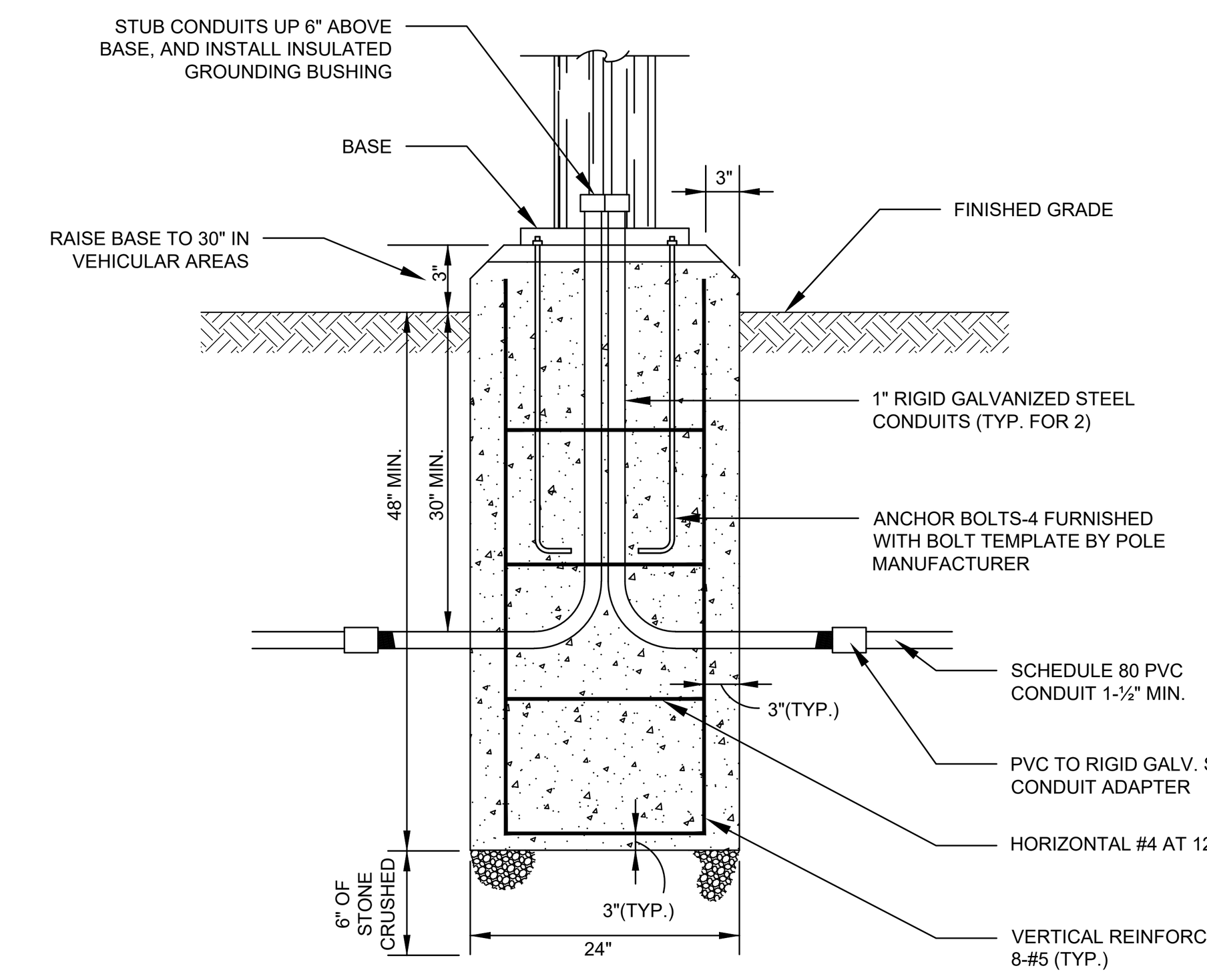


NOTES:

1. ROOF DESIGN LOAD: 4000 LBS. SPREAD OVER ON FOOT SQUARE AREA ANYWHERE ON ROOF.
2. WALLS: SOIL PRESSURE OF EQUIVALENT FLUID PRESSURE OF 33 PCF. SURCHARGE OF 2.5 FEET OF SOIL WEIGHING 120 PCF.
3. CONCRETE: 4000 PSI AT 28 DAYS. ENTRAINED AIR 6-9%
4. STEEL: ASTM A615-1987A, GRADE 40.
5. ALL CONCRETE AND REINFORCEMENT IN ACCORDANCE WITH ACI 318-1986.
6. FOR LIFTING TOP OR BOTTOM SECTIONS, CAST IN FOUR $\frac{3}{4}$ INCH DIAMETER DAYTON SUREGRIP (OR APPROVED EQUAL) COIL LOOP INSERTS GALVANIZED, WITH T21 PLASTIC SETTING PLUGS. INSERTS ARE TO BE SECURED IN PLACE WITH REBAR.
   TOP: CATALOG TYPE B16, $\frac{3}{4}$ INCH DIAM. x 4 INCHES LONG
   BOTTOM: CATALOG TYPE B16, $\frac{3}{4}$ INCH DIAM. x 6 INCHES LONG
7. PROVIDE 3 INCH LONG GROOVE ($\frac{3}{4}$ INCH x 1 INCH) FOR LIFTING SLING AT EACH CORNER, EACH SIDE.
8. MANUFACTURER'S IDENTIFICATION AND MONTH/YEAR WHEN MANUFACTURED SHALL BE LEGIBLY MARKED IN/ON CONCRETE IN THE SIDE.
9. ZINC ALLOY INSERTS $\frac{3}{4}$ INCH - 10 INCHES x 3 INCHES FOR CABLE PULLING TO BE LOCATED 4 INCHES ABOVE (7 INCH x 13 INCH) KNOCKOUTS (4).

PAD-PRECAST CONCRETE-THREE PHASE TRANSFORMER

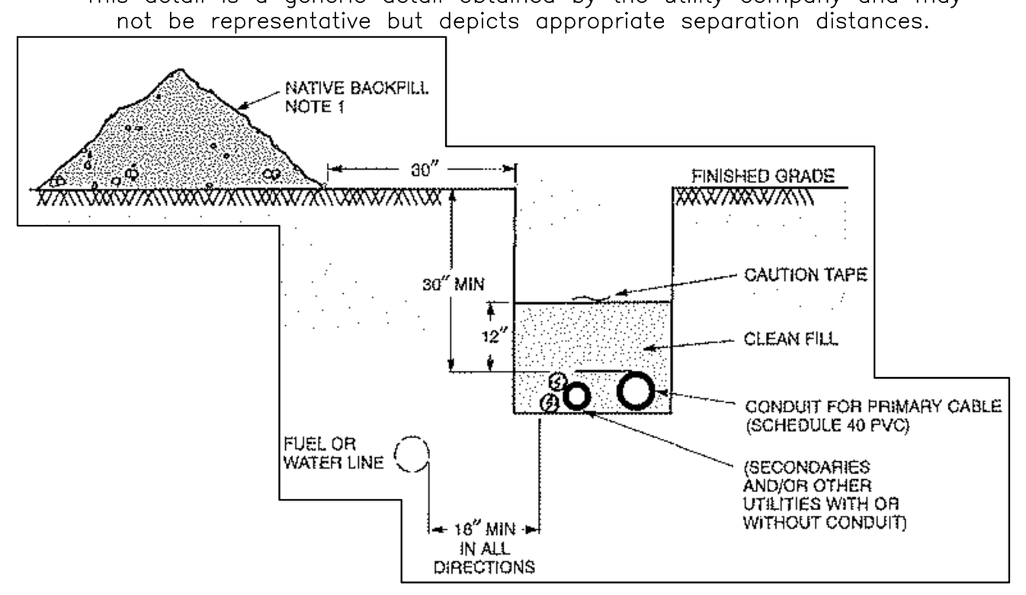
N.T.S.



LIGHT FIXTURE BASE DETAIL

N.T.S.

This detail is a generic detail obtained by the utility company and may not be representative but depicts appropriate separation distances.



BACKFILL:
MATERIAL FOR BACKFILLING SHALL BE EARTH MATERIALS ENTIRELY FREE FROM VEGETATION, TRASH, LUMBER, FROZEN, SOFT OR ORGANIC MATERIALS. NO STONES OR ROCK LARGER THAN THE SIZES LISTED BELOW WILL BE PERMITTED IN THE BACKFILL:

- COMMON FILL—TYPE A: NO STONES OR ROCKS LARGER THAN 1"
- COMMON FILL—TYPE B: NO STONES OR ROCKS LARGER THAN 4"

COMMON FILL MATERIAL MAY BE OBTAINED FROM THE TRENCH EXCAVATION PROVIDED IT HAS BEEN APPROVED BY THE ENGINEER AND HAS BEEN TESTED IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:

A. ALL MATERIALS TO BE USED FOR BACKFILL, INCLUDING COMMON FILL AND BEDDING MATERIALS, SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACING THE MATERIALS IN THE PIPE TRENCH. ALL BACKFILL AND BEDDING MATERIALS WHETHER OBTAINED FROM THE TRENCH EXCAVATION OR FROM AN OFF—SITE SOURCE MUST BE TESTED AS DIRECTED BY THE ENGINEER.
B. SAMPLES OF THE MATERIALS SHALL BE SUBMITTED TO AN APPROVED TESTING AGENCY FOR ANALYSIS. THE TEST RESULTS AND REPORT STATING THAT THE MATERIALS MEET THE REQUIREMENTS THESE SPECIFICATIONS AND THE SPECIFICATIONS OF FEDERAL, STATE AND LOCAL AUTHORITIES (WHERE APPLICABLE) SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO PLACING THE MATERIALS IN THE PIPE TRENCH.

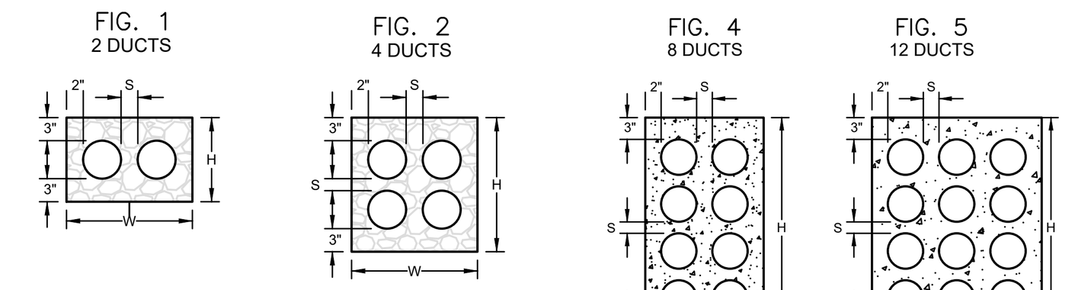
NOTES

1. Minimum cover from top of a conduit bank to the pavement or earth surface to be 36".
2. Duct bank shall extend beyond the property line and capped. Exact location of termination are per field direction. Allow for 20' deviation from locations shown on this plan.
3. Ducts shall be Schedule 40 pvc. Use premanufactured spacers between conduits as necessary. Bends shall be sweeps, 4" "C" Duct telephone bends meeting GTE 8343, United CHS—71 and NEMA TC—10 Specifications.
4. Slope all conduit to drain toward manholes and away from structures.
5. All work shall be performed according to utility company requirements.
6. Ensure that the bottom of the trench is well—tamped and free of rocks
7. Install the conduit, gluing and all couplings
8. Install secondaries and other utility cables or conduits in the trench.
9. Backfill with 12 inches clean fill not to contain stones larger than 4 inches in maximum diameter.
10. Install cable warning.
11. Fill in the remainder of the trench with native backfill
12. Install pull line, including 10 feet of slack, and secure to conduit plug at each end of conduit run.
13. All underground conduit to schedule 40 PVC conduit.
14. Actual utility layout may vary depending on final utility company coordination. Coordination of final layout shall be the contractor's responsibility.
15. All underground utilities crossing a roadway shall be concrete encased.
16. Concrete encasement shall be color red within the limits of the state right—of—way.

	ALL DIMENSIONS IN INCHES											
	2" DUCT			3" DUCT			4" DUCT			5" DUCT		
FIG	W	H	S	W	H	S	W	H	S	W	H	S
1	10 1/2"	8 1/2"	1 1/2"	12 1/2"	9 1/2"	1 1/2"	14 1/2"	10 1/2"	1 1/2"	16 1/2"	11 1/2"	1 1/2"
1A	6 1/2"	12 1/2"	1 1/2"	7 1/2"	14 1/2"	1 1/2"	8 1/2"	16 1/2"	1 1/2"	9 1/2"	18 1/2"	1 1/2"
2	10 1/2"	12 1/2"	1 1/2"	12 1/2"	14 1/2"	1 1/2"	14 1/2"	16 1/2"	1 1/2"	16 1/2"	18 1/2"	1 1/2"
2A	18 1/2"	8 1/2"	1 1/2"	22 1/2"	9 1/2"	1 1/2"	26 1/2"	10 1/2"	1 1/2"	30 1/2"	11 1/2"	1 1/2"
4	14 1/2"	28 1/2"	1 1/2"	16 1/2"	32 1/2"	1 1/2"	14 1/2"	28 1/2"	1 1/2"	16 1/2"	32 1/2"	1 1/2"
4A	26 1/2"	16 1/2"	1 1/2"	30 1/2"	18 1/2"	1 1/2"	26 1/2"	16 1/2"	1 1/2"	30 1/2"	18 1/2"	1 1/2"
5	14 1/2"	28 1/2"	1 1/2"	16 1/2"	32 1/2"	1 1/2"	20 1/2"	28 1/2"	1 1/2"	23 1/2"	32 1/2"	1 1/2"
5A	26 1/2"	24 1/2"	1 1/2"	30 1/2"	27 1/2"	1 1/2"	26 1/2"	22 1/2"	1 1/2"	30 1/2"	25 1/2"	1 1/2"
6A										30 1/2"	32 1/2"	1 1/2"
7A										30 1/2"	39 1/2"	1 1/2"

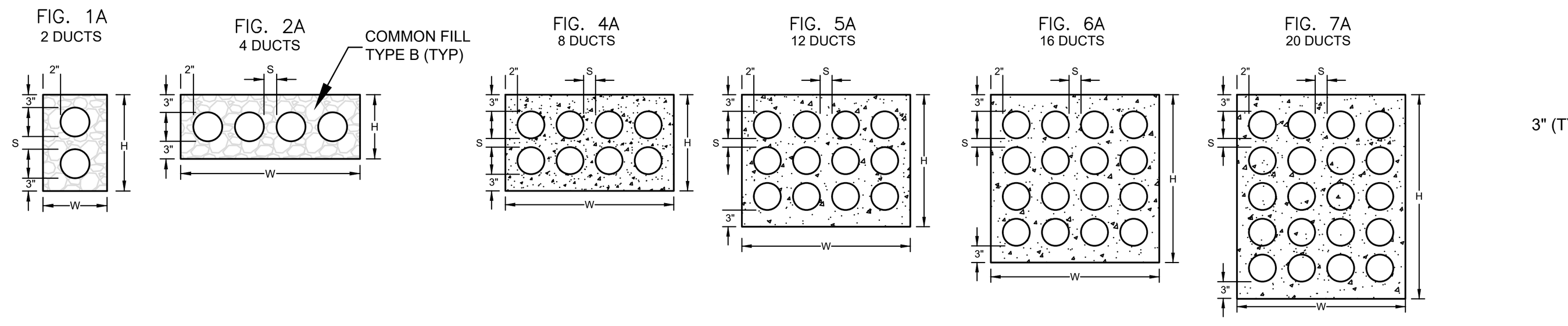
PREFERRED:

FIG. 1 2 DUCTS
FIG. 2 4 DUCTS
FIG. 4 8 DUCTS
FIG. 5 12 DUCTS

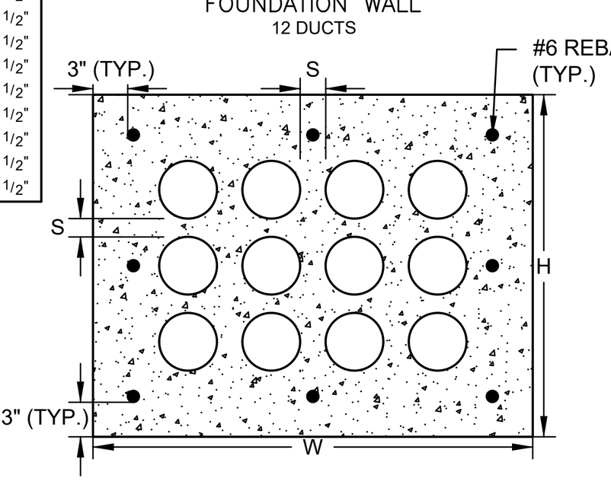


ALTERNATE: (SEE NOTE 4)

FIG. 1A 2 DUCTS
FIG. 2A 4 DUCTS
FIG. 4A 8 DUCTS
FIG. 5A 12 DUCTS
FIG. 6A 16 DUCTS
FIG. 7A 20 DUCTS

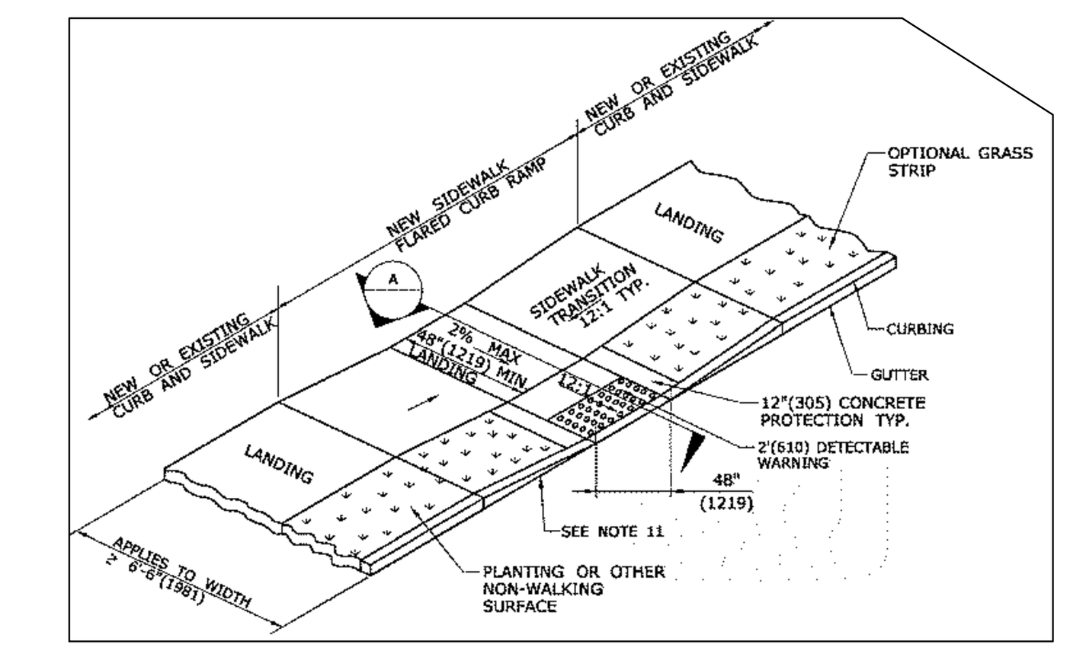


DUCT BANK AT BUILDING FOUNDATION WALL
12 DUCTS



CONDUIT BANK CONSTRUCTION

N.T.S.



GENERAL NOTES:

1. MAXIMUM SLOPES OF ADJOINING GUTTERS AND ROAD SURFACES IMMEDIATELY ADJACENT TO THE SIDEWALK RAMP OR ACCESSIBLE ROUTE SHOULD NOT EXCEED 20:1.
2. CARE SHALL BE TAKEN TO ASSURE UNIFORM GRADE ON THE RAMP, FREE OF SAGS AND ABRUPT GRADE CHANGES.
3. ALL RAMPS SHALL BE CONSTRUCTED OF CLASS "C" CONCRETE IN ACCORDANCE WITH CONNECTICUT STANDARD SPECIFICATIONS ARTICLE M.03.01.
4. SIDEWALK RAMPS SHALL HAVE A COARSE BROOM FINISH TRANSVERSE TO THE SLOPE OF THE RAMP.
5. DIAGONAL SIDEWALK RAMPS AT MARKED CROSSINGS SHALL BE WHOLLY CONTAINED WITHIN THE MARKINGS, EXCLUDING ANY FLARED SIDES.
6. REMOVAL OF EXISTING SIDEWALK FOR NEW RAMP INSTALLATIONS SHALL BE TO THE NEAREST EXPANSION / CONTRACTION JOINT OR DUMMY JOINT. 12:1 MAY NOT BE ACHIEVABLE DUE TO SIDEWALK GRADE. IN RECOGNITION OF THIS, A MINIMUM LIMIT OF 15' (4.57m) FOR A PARALLEL RAMP SHALL BE USED. REMOVAL SHALL NOT BE FURTHER THAN 2' (610) FROM THE PROPOSED RAMP UNLESS DIRECTED BY THE ENGINEER. SAW CUT REQUIRED FOR DUMMY JOINTS SHALL BE INCLUDED IN THE COST OF "CONCRETE SIDEWALK".
7. EXPANSION JOINTS IN CONCRETE SHALL MATCH THOSE IN ADJACENT SIDEWALKS BUT IN NO CASE SHALL THE SPACING BETWEEN EXPANSION JOINTS EXCEED 12' (3.66m) UNLESS OTHERWISE NOTED.
8. RAISED MEDIAN ISLANDS IN MARKED CROSSINGS SHALL HAVE SIDEWALK RAMPS AT BOTH SIDES AND A LEVEL AREA AT LEAST 4' (1219) LONG BETWEEN THE RAMPS. IF THIS CAN NOT BE ACHIEVED, THE RAISED ISLAND SHALL BE CUT THROUGH LEVEL WITH THE ROADWAY AS SHOWN ON THE PLANS OR AS DIRECTED BY THE ENGINEER.
9. SIDEWALK RAMPS, WHEN CONSTRUCTED IN CONJUNCTION WITH SIDEWALK, SHALL BE PAID FOR UNDER THE ITEM "CONCRETE SIDEWALK", INCLUDING CURBING WITHIN THE LIMITS OF THE NEW SIDEWALK RAMP AND DETECTABLE WARNING STRIPS. WHEN JUST A SIDEWALK RAMP IS CONSTRUCTED, IT SHALL BE PAID FOR UNDER THE ITEM "SIDEWALK RAMP" INCLUDING CURBING WITHIN THE LIMITS OF THE NEW SIDEWALK AND DETECTABLE WARNING STRIP.
10. CURBING WITHIN THE LIMITS OF THE NEW SIDEWALK RAMP SHALL BE CONSTRUCTED IN CONFORMANCE WITH THE REQUIREMENTS OF FORM 816 SECTIONS 8.11 AND 8.13.
11. TRANSITION TO FULL HEIGHT CURB. INSTALL STONE CURBING IF ADJACENT CURBING IS STONE. INSTALL CONCRETE CURBING IF ADJACENT CURBING IS CONCRETE OR BITUMINOUS.
12. INSTALL THE EDGE OF THE DETECTABLE WARNING STRIP 6" (152) FROM THE EDGE OF ROAD.
13. TO PERMIT WHEELCHAIR WHEELS TO ROLL BETWEEN DOMES, ALIGN DOMES ON A SQUARE GRID IN THE DIRECTION OF PEDESTRIAN TRAVEL.

PEDESTRIAN RAMP DETAIL

N.T.S.

GENERAL
1. The customer (contractor) shall be responsible for service trench, conduit, concrete encasement and conduit inspections.
2. NU shall be responsible for sealing the inside of the conduit.
3. NU shall not be responsible for any leak between the conduit and the wall.

SERVICE TRENCH – Trench location, as specified by NU, shall be in as direct a line as possible without reverse curves from the distribution facility to the customer service entrance.

Trench shall be excavated and backfilled by the customer.

Corrosive fill such as cinders shall not be used.

The backfill within 6 inches of conduit shall not contain any large or sharp rocks or other objects that might damage conduit.

The trench shall have a 24–inch minimum cover over supply conduit to finish grade, except where ledge is encountered, then the cover may be reduced to 18 inches if steel is used.

The trench shall have a 4–inch–per–100–foot downward pitch toward distribution facility, if physically possible.

Maintain a 12–inch minimum separation from other facilities except for communication conduit which may have 3 inches of concrete separation.

CONDUIT – Conduit shall be as specified by NU but supplied and installed by customer.

	Steel Galv	IMC	PVC Schedule 40	PVC Type EB*
Direct–Buried (DB)	X	X	X	X
Disturbed Earth (i.e., Filled Area)	X	X	–	X
Delta Primary (i.e., 4.8 kV)	X	X	–	X
*Must be encased in concrete				

For a discussion of the types of conduit and their applications, see **DTR 44.351**.

Sweeps in the conduit run, achieved by forcing a gradual bend in a length of Type EB PVC conduit, shall have a minimum radius of 15 feet. Manufactured bends in the conduit run shall have a minimum radius of 48 inches. This requirement does not include the bends used at riser poles or equipment pads where the bend radius shall be a minimum of 24 inches, with 36 inches preferred.

There must be a seal between conduit and building wall.

CONCRETE ENCASEMENT – Concrete shall be 2,000 psi, 28 day strength with 1/2–inch maximum aggregate. A stiff field mix of 1 part cement, 3 parts sand, 5 parts stone (1:3:5) will be acceptable.

Encasement shall be 3 inches top and bottom, 2 inches sides and 1–1/2 inches between conduits (except 2 inches between 6–inch conduit). All dimensions are minimum.

When steel conduit and PVC conduit are joined the encasement should be extended 1 foot onto the steel conduit.

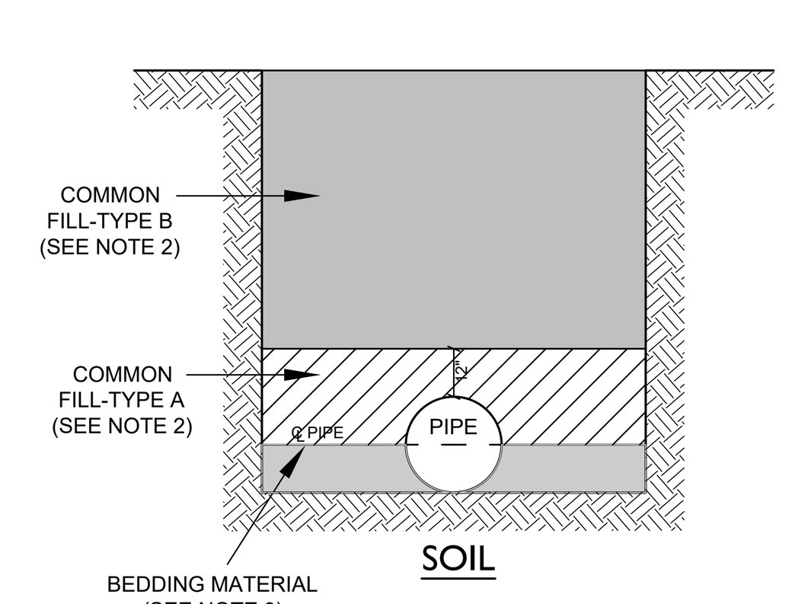
CONDUIT INSPECTION
1. Conduit(s) shall be cleaned with a wire brush of the same diameter as the conduit.
2. A test shall be made by pulling a 17–inch–long flexible mandrel through the conduit, equal to diameter of the conduit. NU reserves the right to witness the cleaning and testing.
3. A 1/4–inch–diameter nylon pull line shall be placed in the conduit, including 10 feet of slack, and secured to a plastic conduit plug at each end of the conduit run.

CONDUIT SEALING – Conduit occupied with cable is to be sealed by NU at the customer service entrance with jute and duct sealing putty. The water–path in bare standard neutral cable will be sealed by splicing a piece of covered cable onto the bare neutral using a waterstop connector (See **DTR 73.251–.252**).

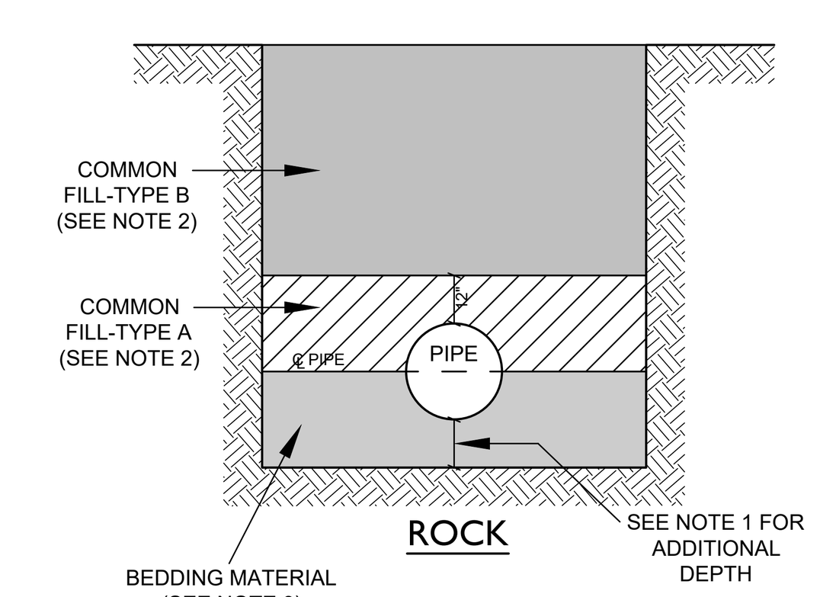
Empty conduit shall be sealed at the customer service entrance with a plastic plug to prevent the possible entry by water or gas. If physical conditions require conduit to slope toward the customers facilities additional seals will be required at the distribution facilities, i.e., manhole or other types of UG structures.

EVERSOURCE CONDUIT INSTALLATION DETAIL

N.T.S.



SOIL



ROCK

NOTES:
1. THE TRENCH SHALL BE EXCAVATED TO THE DEPTH REQUIRED, SO AS TO PROVIDE A UNIFORM AND CONTINUOUS BEARING AND SUPPORT FOR THE PIPE BARREL ON SOLID AND UNDISTURBED GROUND AT EVERY POINT BETWEEN JOINTS, EXCEPT THAT IT WILL BE PERMISSIBLE TO DISTURB THE FINISHED TRENCH BOTTOM OVER A MAXIMUM LENGTH OF 18" NEAR THE MIDDLE OF EACH LENGTH OF PIPE BY THE WITHDRAWAL OF PIPE SLINGS OR OTHER LIFTING TACKLE. WHEN REQUIRED, BELL HOLES SHALL BE PROVIDED. THE FINISHED TRENCH BOTTOM SHALL BE ACCURATELY PREPARED BY MEANS OF HAND TOOLS.
2. MATERIAL FOR BACKFILLING SHALL BE EARTH MATERIALS ENTIRELY FREE FROM VEGETATION, TRASH, LUMBER, FROZEN, SOFT OR ORGANIC MATERIALS. NO STONES OR ROCK LARGER THAN THE SIZES LISTED BELOW WILL BE PERMITTED IN THE BACKFILL:

   COMMON FILL-TYPE A: NO STONES OR ROCKS LARGER THAN 1"
   COMMON FILL-TYPE B: NO STONES OR ROCKS LARGER THAN 4"

   COMMON FILL MATERIAL MAY BE OBTAINED FROM THE TRENCH EXCAVATION PROVIDED IT HAS BEEN APPROVED BY THE ENGINEER AND HAS BEEN TESTED IN ACCORDANCE WITH THE FOLLOWING REQUIREMENTS:

   a.a. ALL MATERIALS TO BE USED FOR BACKFILL, INCLUDING COMMON FILL AND BEDDING MATERIALS, SHALL BE APPROVED BY THE ENGINEER PRIOR TO PLACING THE MATERIALS IN THE PIPE TRENCH. ALL BACKFILL AND BEDDING MATERIALS WHETHER OBTAINED FROM THE TRENCH EXCAVATION OR FROM AN OFF-SITE SOURCE MUST BE TESTED AS DIRECTED BY THE ENGINEER.
   a.b. SAMPLES OF THE MATERIALS SHALL BE SUBMITTED TO AN APPROVED TESTING AGENCY FOR ANALYSIS. THE TEST RESULTS AND REPORT STATING THAT THE MATERIALS MEET THE REQUIREMENTS THESE SPECIFICATIONS AND THE SPECIFICATIONS OF FEDERAL, STATE AND LOCAL AUTHORITIES (WHERE APPLICABLE) SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO PLACING THE MATERIALS IN THE PIPE TRENCH.

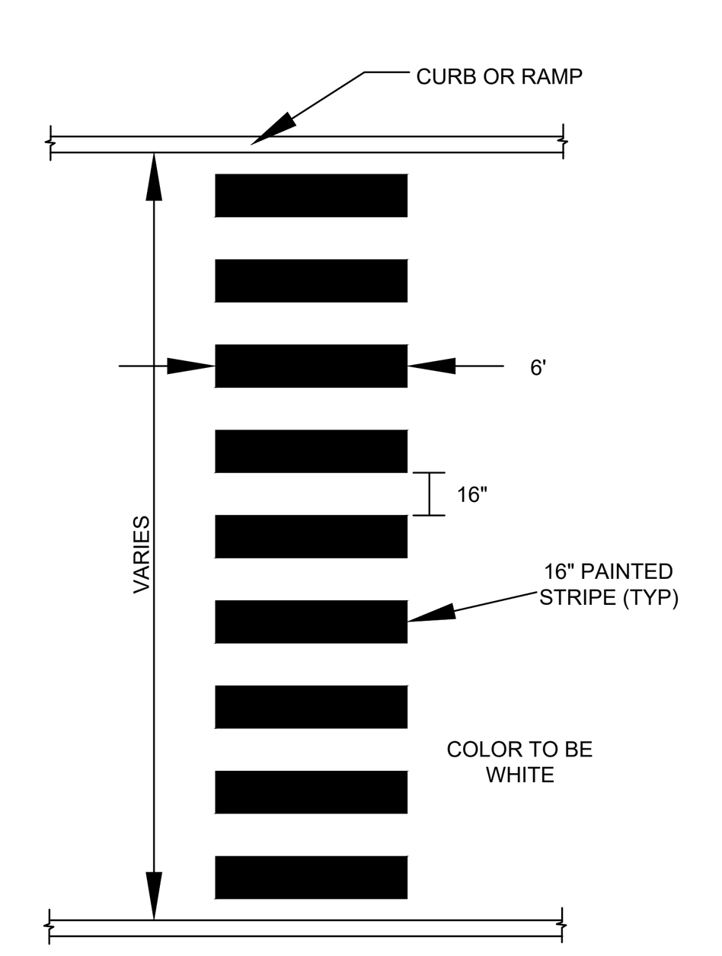
   IF APPROVED MATERIAL OBTAINED FROM THE TRENCH EXCAVATON IS INSUFFICIENT TO COMPLETE THE BACKFILL, THE CONTRACTOR SHALL OBTAIN THE NECESSARY APPROVED COMMON FILL MATERIALS FROM AN OFF-SITE SOURCE.
3. MATERIALS USED FOR BEDDING AND THE HAUNCH AROUND THE PIPE SHALL BE A COARSE TO FINE SANDY MATERIAL WITH MAXIMUM STONE SIZE OF 1 INCH. THE MATERIAL SHALL CONFORM TO ASTM D2487 STANDARD METHOD FOR CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES" USING THE UNIFIED SOIL CLASSIFICATION SYSTEM, EXCEPT WHERE A HIGHER STANDARD IS REQUIRED ELSEWHERE IN THE CONTRACT DOCUMENTS OR BY RULES OR REGULATIONS OF FEDERAL, STATE OR LOCAL GOVERNMENTAL BODIES HAVING JURISDICTION OVER THE SITE OF THE WORK.

   THE MATERIAL SHALL MEET A CLASS II DESIGNATION. SOIL TYPES GW, GP, SW AND SP, NON-COHESIVE, WELL GRADED AND CONTAINING SOME FINES ARE INCLUDED IN THIS CLASS. WHERE VOIDS, FINER GRAINED SOILS OR MOVEMENT MAY ALLOW MIGRATION OF THIS MATERIAL, A FILTER FABRIC AS DIRECTED BY THE ENGINEER WILL BE USED IN THE TRENCH BOTTOM AND SIDES BEFORE THE SELECT FILL BEDDING IS PLACED.

   BEDDING MATERIAL MAY BE OBTAINED FROM THE TRENCH EXCAVATION PROVIDED IT HAS BEEN TESTED IN ACCORDANCE WITH THE REQUIRE- MENTS STATED ABOVE AND APPROVED BY THE ENGINEER. IF THE APPROVED MATERIAL OBTAINED FROM THE TRENCH EXCAVATION IS INSUFFICIENT TO COMPLETE THE BEDDING, THE CONTRACTOR SHALL OBTAIN THE NECESSARY TESTED AND APPROVED BEDDING MATERIALS FROM AN OFF-SITE SOURCE.
4. REFER TO SECTION 2210 OF THE CONNECTICUT-AMERICAN WATER COMPANY SPECIFICATIONS.

TRENCH BACKFILL MATERIALS (WATER LINE)

N.T.S.



CROSSWALK

N.T.S.

No.	Date	Revision
4	05/15/2023	REVISED PER PZC COMMENTS
3	02/28/2023	REVISED PER BUILDING DESIGN
2	01/02/2023	REVISED PER DPW COMMENTS
1	09/30/2022	ORIGINAL ISSUE DATE

DETAILS
DEPICTING
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CRAIG J. FLAHERTY CT. P.E. 21149

May 15, 2023

DATE

SHEET No: SE-4

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